Prepared for: Superior Water Light & Power Company Superior, Wisconsin



Remedial Action Options Report Superior Water Light & Power MGP Superior, Wisconsin WDNR BRRTS #02-16-275446

ENSR Corporation February 5, 2008 Document No.: 09413-098



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1.0 Introduction

ENSR Corporation (ENSR) has prepared the following Remedial Action Options Report for the Superior Water Light & Power (SWL&P) Former Manufactured Gas Plant (MGP), located at the intersection of Winter Street and East 1st Street in Superior, Wisconsin (site). The site location is shown in **Figure 1**. This report has been prepared in accordance with the Wisconsin Administrative Code Chapter NR 722.

1.1 Background

1.1.1 Site History

The gas plant was built in 1888 and began operations on November 1, 1889. The gas plant produced carbureted water gas made by the improved "Springer" process. Two gas holders were initially constructed on the site: one single lift holder of 35,000 cubic feet capacity, built in October 1889, and one double lift holder of 250,000 cubic feet capacity, dimensions of 92 ft x 21 ft x 21 ft, completed in October 1891. In 1924, a third gas holder was constructed at the site. This 750,000-cubic foot gas holder was located southwest of the former MGP building. A spherical gas holder called the "Horton Sphere" was constructed in 1950.

Gas was produced at the Superior MGP from November 1889 to August 1904. After August 1904, all gas sold by SWL&P was purchased from the Zenith Furnace Company (later known as Interlake Corporation). The gas purchased from Zenith/Interlake was purified in West Duluth before it was piped to SWL&P's plant in Superior. Therefore, no purifier wastes were generated at the site after August 1904. The MGP at the site produced a total of approximately 262,000 MCF (million cubic feet) of gas during its 15-year production history.

In 1929, the gas plant building was rebuilt to its present configuration. Gas purchased from Zenith/Interlake was stored in the gas holders, and pumped and metered from the reconstructed building. Storage and metering of manufactured gas purchased from Zenith/Interlake continued until natural gas supplies became available in 1959. The 35,000-cubic foot gas holder was removed prior to 1938. The 250,000-cubic foot gas holder was removed between 1940 and 1961. The 750,000-cubic foot gas holder was removed between 1942 and 1964, and the Horton Sphere was removed in 1985.

In 1978, SWL&P sold the former gas plant building and portions of the property to CLM, Inc. The building was gutted, concrete floors were poured over the existing sand floors, and the building has been used for storage since that time.

1.1.2 Previous Investigations

ENSR conducted a Phase I environmental assessment at the site for SWL&P in September/October of 2001. The Phase I report indicated that gas was manufactured at the site for fifteen years, ending in 1904. Areas of the site that had potential to contain MGP-related chemicals and/or byproducts were identified as part of the Phase I assessment. ENSR performed a Phase II site investigation from November 2001 through February 2002. Results of the Phase II indicated areas of the site contained volatile organic compounds (VOCs) and polycyclic aromatic hydrocarbons (PAH) compounds in the soil above Wisconsin Department of Natural Resources (WDNR) Residual Contaminant Levels (RCL). Groundwater samples contained benzene, toluene, ethylbenzene, and xylene (BTEX) and PAH compounds above the WDNR groundwater Enforcement Standard (ES). South of the site building, an area contained exclusively BTEX of unknown origin. Other site areas contained both PAH and BTEX. The former gas holder tank bases were investigated with a backhoe. MGP wastes were not found in association with the tank bases. Soil containing tarry residues and PAH were found in the area between the former site building and the 1904-era shoreline.

A Phase II, Part II Investigation was completed in September 2002 to further delineate the PAH and BTEX in the soil and groundwater. Fingerprinting analytical results indicated the VOCs found south of the site building (near the former Horton Sphere gas holder) appeared to be a blended solvent or degreaser consisting of primarily benzene and toluene with lesser amounts of ethylbenzene and xylene. In addition, a test trench excavation north of the site building encountered a clay tile pipe oriented toward the former Superior Bay shoreline that contained tarry material. The tarry material was analyzed using "fingerprinting" techniques and appeared to be carbureted water gas coal tar. Soil at the end of the clay pipe contained some tar masses as well as residual tar in surrounding soils. The results of the Phase II, Part II investigation indicated additional BTEX and PAH impacts downgradient of the site.

A sediment investigation was completed in the Superior Bay boat slip and nearby storm sewers in March and April 2003. The sediment results indicated concentrations of PAH similar to typical urban run-off in the storm sewer and boat slip. Thus, the investigation shifted back to soil and groundwater in upland areas of the site.

The Phase II Part III subsurface investigation was completed in October and November 2004. The investigation consisted of installing eight soil borings, B-24 through B-31, and five monitoring wells, MW-8 through MW-12, located off-site to the north and east of the site. The results indicated that the plume of dissolved BTEX and PAH in the groundwater followed the groundwater flow direction and are found off-site northeast and east of the site.

Soil containing tarry residues was observed while drilling well MW-8. The MW-8 boring did not encounter tarry masses or mobile tar. None of the wells at the site have collected mobile tar (no measurable free product). The tarry residues appeared to extend from the terminus of the clay pipe on-site to the southeast towards MW-8, in the area along the former shoreline. Tarry residues were not found in the area between the railroad tracks and Superior Bay. However, additional investigation was needed to determine the extent of the tarry residues.

The Phase II Part IV subsurface investigation was completed in 2005 and 2006 to delineate the extent of tarry residues in the subsurface, and the extent of downgradient VOC and PAH groundwater impacts. The May 2007 Phase II Part IV report summarizes the results of the investigation and a site specific human health risk assessment. **Figure 2** shows the sample locations for the investigations conducted at the site. The following site work was completed as part of the Phase II Part IV investigation:

- Completed membrane interface probe (MIP) borings for VOC delineation in September 2005.
- Completed groundwater VOC plume delineation using mobile on-site laboratory in September 2005.
- Completed first round of Tar-specific Green Optical Screening Tool (TarGOST[™]) borings for coal tar delineation in October 2005.
- Installed monitoring wells MW-13 through MW-22 in October 2005.
- Collected groundwater samples from all site monitoring wells in November 2005.
- Completed second round of TarGOST borings in May 2006.
- Completed two TarGOST confirmation Geoprobe soil borings in May 2006.
- Completed surface soil sampling in May 2006.
- Collected groundwater samples from MW-13 through MW-22 in October 2006.
- Completed slug tests on wells MW-15 through MW-22 in October 2006.

The Phase II Part IV Investigation completed the delineation of VOC and PAH in the soil and groundwater. The results of the human health risk assessment indicated potential excess risk on the former MGP property for the soil to ambient air, soil to indoor air, and direct contact exposure scenarios.

1.2 Objectives

The purpose of this report is to identify and evaluate likely remedial action options for "source" soil (soil that contains tarry residues and has potential excess risk) and selects the most appropriate (cost effective and technically feasible) option. Once source soils have been addressed, monitored natural attenuation is expected to address dissolved contaminants in groundwater. This report includes:

- A summary of the nature and extent of impacted soil and groundwater;
- A summary of the geological and hydrogeological conditions across the study area;
- A review of applicable remedial action options for the Site; and
- An overview of the selected remedial action.

1.3 Site Location, Ownership and Consultant/Contractor Information

The former Superior MGP Site is located in the vicinity of the intersection of Winter Street and East 1st Street in Superior, Douglas County, Wisconsin. The Site occupies a portion of the northeast quarter of the northwest quarter of Section 13, Township 49 North and Range 14 West (SW ¼, NW ¼ of Sec. 13, T49N, R14W). The Site location is depicted on Figure 1.

Portions of the former MGP property are now owned by Superior Water Light & Power (SWL&P), the City of Superior, the U.S. Department of Transportation, and CLM, Inc. **Figure 3** is a color-coded map indicating property ownership in the vicinity of the MGP Site.

The owner contact is:

Bill Bombich Superior Water Light and Power Company 2915 Hill Avenue Superior, Wisconsin 54880 (715) 395-6288

ENSR has been retained as the consultant at the Site. The consultant information is as follows:

ENSR Corporation Attn: William M. Gregg 4500 Park Glen Road, Suite 210 St. Louis Park, MN 55416 (952) 924-0117 - phone (952) 924-0317 - fax bgregg@ensr.aecom.com

1.4 Nature and Extent of Soil Impacts

1.4.1 Surface Soil

Surface soil (soils within four feet of ground surface) samples were collected as part of the Phase II Part IV Investigation from areas of the site where the MGP formerly operated. Surface soil samples were also collected from soil borings, well borings, and test trenches during previous investigations. VOC surface soil analytical results were compared to the Protection of Human Health from Direct Contact with Contaminated Soil, Table 2, NR 746 criteria. PAH soil analytical results were compared to Soil Cleanup Levels of Polycyclic Aromatic Hydrocarbons Interim Guidance, WDNR Publication RR-519-97, Industrial Direct Contact Pathway. Based on a review of the analytical results (**Table 1**), surface soil is impacted above the criteria in the following locations:

- Samples T8-S1, B-15-1-3, SS-5, SS-10, and SS-14 contained concentrations of benzene above the NR 746 direct contact criteria. The benzene detections are likely due to separate sources in each of the sample locations, as opposed to a large contiguous source. Prior investigations have found MGP residues and/or a benzene-rich solvent at these sample locations.
- PAH were found in several of the surface soil samples at concentrations above the WDNR PAH Interim guidance direct contact criteria including B-15-1-3, MW-20, SS-2, SS-5, SS-6, SS-7, SS-8, SS-10, SS-13, SS-19, T1-S1, and T9-S2. Most of these samples were located in the gravel yard around the former MGP building.

1.4.2 Subsurface Soil

Laboratory analytical results indicated that subsurface soils (soils at depths greater than four feet below ground surface) are also impacted by VOC and PAH. The subsurface soil VOC results were compared to the Residual Contaminant Level for Protection of Groundwater, Table 1, Soil Cleanup Standards, NR 720. PAH subsurface soil analytical results were compared to Soil Cleanup Levels of Polycyclic Aromatic Hydrocarbons Interim Guidance, WDNR Publication RR-519-97, Protection of Groundwater Pathway. Subsurface soil sample analytical results are summarized in **Table 2**. The BTEX and several PAH concentrations in on-site and off-site soil samples exceeded the protection of groundwater criteria.

1.4.3 Risk Assessment Results

Based on the results of the Human Health Risk Assessment included in the Phase II Part IV report, concentrations of VOC in subsurface and surface soils potentially pose excess risk for the volatilization to ambient air and indoor air inhalation pathway. The following is a summary of the areas on the former MGP property with excess risk:

- Benzene in surface and subsurface soil (soil to ambient air modeling). The samples and areas at the former MGP with the highest concentrations of benzene in soil are located near or north of the former MGP building (B-11, B-12, B-13, B-14, B-15, B-16, B-17, MW-7, T-9) or south of East 1st Street near the former Horton Sphere (B-8, B-10, MW-4, T-8). The concentration at MW-7 (maximum detect) was used as the benzene exposure point concentration for ambient air modeling.
- 1,3,5-trimethylbenzene in surface and subsurface soil (soil to ambient air modeling). The location with the highest 1,3,5-trimethylbenzene concentration is B-15, north of the former MGP building.
- Benzene in surface and subsurface soil (soil to indoor air modeling). The concentration at T9-S2 (maximum detect near building) was used as the benzene exposure point concentration for ambient air modeling.
- Benzo(a)pyrene in surface soil (direct contact). The concentration at sample location B-13, (23 mg/kg) dominates the risk calculation.

The use of maximum concentrations likely overestimates the potential risks. The risk assessment identified areas and chemicals of potential concern that may warrant further investigation, advanced risk assessment, and/or remediation. These include benzene and 1,3,5-trimethylbenzene in soil near and north of the former MGP building and benzo(a)pyrene in surface soil at the site.

1.4.4 Tarry Residues

A test trench excavation north of the MGP building encountered a clay tile pipe oriented northeast toward the former shoreline that contained tarry masses and residues. The tarry masses were analyzed using fingerprinting techniques and appeared to be carbureted water gas coal tar. Soil borings and test trenches revealed tarry residues oriented along the former shoreline. The TarGOST borings also confirmed that the highest concentrations and thickest deposits occur near the terminus of the clay pipe. This area is interpreted to be the "source area" for dissolved PAH in groundwater and tarry residues found in downgradient areas. The physical appearance and distribution of tarry residues are consistent with the discharge of wastewater from the clay pipe and subsequent transport and deposition along the former shoreline. Using the TarGOST, the tarry residues were delineated as illustrated on **Figure 4**.

No free product has been measured in the monitoring wells at the site. The MGP residue detected by the TarGOST does not appear to be mobile, but is a source of dissolved PAH and VOC in the groundwater. The tarry residue appears to be adsorbed to the soil matrix where it was historically deposited based on the following observations:

- The tarry residue was found within the fill material and has not migrated downward to the clay basal unit or to other lower permeable materials in most locations.
- Measurable free product has not accumulated in monitoring wells completed within the tarry residues (such as wells MW-7 or MW-8).
- The tarry residues do not follow groundwater flow direction and appear to be located where they were historically deposited. The tarry residues do not appear to be migrating.

1.5 Nature and Extent of Groundwater Impacts

The groundwater analytical results were compared to Wisconsin NR 140 Enforcement Standards (ES). Groundwater analytical results from the most recent groundwater monitoring event are summarized in **Table 3**. As shown in Table 3, VOC and PAH concentrations exceed the ES. The extent of dissolved PAH in groundwater was delineated to the ES criteria as illustrated on **Figure 5**. The location of the dissolved PAH plume with concentrations above the ES appears to be similar to the location of tarry materials delineated by the TarGOST borings. The majority of VOC soil and groundwater impacts were found commingled with the tarry residues and PAH plume. The extent of the dissolved VOC plume in groundwater at concentrations above the ES is illustrated on **Figure 6**.

Dissolved VOC and PAH are present downgradient from the former shoreline source area, migrating with groundwater through an aquifer that consists of fill materials. Other isolated areas of PAH and VOC, such as near the former gas holders, do not appear to be migrating due to the clay soils at those locations.

1.6 Chemicals of Potential Concern

Chemicals detected in the soil and groundwater at the site during the previous site investigation activities were screened against applicable standards. Chemicals that were detected at concentrations greater than or equal to the applicable standard were considered chemicals of potential concern (COPCs). Additionally, chemicals that were detected in greater than ten percent of the samples, but for which there were no applicable standards, were also considered COPCs. The site investigation identified the following COPCs:

1,2,4-Trimethylbenzene (soil only)	Benzo(a)pyrene
1,3,5-Trimethylbenzene (soil only)	Benzo(b)fluoranthene
Benzene	Benzo(ghi)perylene
Ethylbenzene	Benzo(k)fluoranthene
Toluene	Chrysene
Xylenes	Dibenz(a,h)anthracene
1-Methylnaphthalene	Fluoranthene
2-Methylnaphthalene	Fluorene
Acenaphthene	Indeno(1,2,3-cd)pyrene
Acenaphthylene	Naphthalene
Anthracene	Phenanthrene
Benzo(a)anthracene	Pyrene

1.7 Lithological, Geological and Hydrogeological Site Characteristics

1.7.1 Geology

The MGP site is located at an elevation between 610 and 615 feet above mean sea level. The topography of the former MGP is relatively flat with little or no slope. To the northeast of the former MGP, the topography slopes down towards the railroad tracks. The land surface to the north of the railroad tracks is relatively flat with most elevations between 605 and 607 feet above mean sea level. The water elevation in Superior Bay is approximately 601 feet above mean sea level.

Aerial photographs and historic maps of the City of Superior, obtained during the Phase I, indicate the former Superior Bay shoreline was originally located approximately 50 to 75 feet northeast of the MGP building. Water was present between the former shoreline and the railroad track causeway in Superior Bay (referred to as the "pond" in historic documents). By 1905, the area between the former shoreline and the railroad tracks had been filled, and no water was present. Various shoreline development and filling activities continued between 1905 and 1978. The 1978 aerial photo depicts the area north of the railroad tracks in its current configuration.

The results of the subsurface investigations indicate that there are several predominant soil types encountered in the area:

- Reddish-brown high-plasticity clay;
- Sand and silty sand;
- Fill material consisting primarily of light gray to dark gray lime-like material; and
- Miscellaneous fill such as bricks, wood, slag, and cinders.

According to the Bedrock Geology of Wisconsin map, sandstone bedrock (Keweenawan Formation) may be found beneath the unconsolidated soils. Depth to bedrock is estimated to be from 100 to 200 feet below the ground surface. (Wisconsin Geological and Natural History Survey http://www.uwex.edu/wgnhs/bdrk.htm)

During previous investigations, several borings were installed at the City of Superior wastewater treatment plant (WWTP) located east of the site. The WWTP was constructed on fill placed in Superior Bay and the borings encountered primarily sandy soil. Dredge spoils were likely used to create the land for the WWTP. Wood waste and saw dust are also present in this area owing to the former use of this property as a saw mill.

Borings installed along the south side of the rail road tracks and in the gravel parking area north of the MGP building encountered lime-like fill material as the uppermost soil type. Underlying the lime-like fill material was silty sand along with miscellaneous fill (slag, wood, brick, etc.) in some borings. Underlying the sand unit or miscellaneous fill was reddish-brown high plasticity clay. The elevation of the clay unit appears to slope northeast and east-northeasterly, towards Superior Bay. Clay was encountered at the ground surface in the borings located southwest of the MGP building. **Figure 7** illustrates the elevation of the clay soil.

1.7.2 Hydrogeology

North of the MGP building, groundwater was encountered in the sand, silty sand, or fill material above the red clay. Groundwater was encountered approximately two to five-feet below the ground surface in the wells along the railroad track right-of-way and to the north. Groundwater was approximately eight to eleven-feet below the ground surface in the wells south of the railroad tracks. Depth to groundwater was gauged prior to collecting groundwater samples from the monitoring wells. The gauging data results from October 2006 are summarized in **Table 4**. Groundwater elevation contours from October 2006 are illustrated on **Figure 8**. The apparent groundwater flow direction at the site appears to be northeast towards Superior Bay.

Results of the slug tests performed on the monitoring wells indicate hydraulic conductivity of the aquifer ranged from 10^{-5} centimeters per second (cm/s) to 10^{-3} cm/s in wells installed in the lime-like material and 10^{-3} to 10^{-1} cm/s in wells installed in the sand unit. Slug tests were not completed on wells screened in the clay soil because of the slow recharge rate. For example, static water levels were reached in wells MW-13 and MW-14 several months after installation. Hydraulic conductivity of the clay unit is estimated to be less than 10^{-6} . Hydraulic conductivity values are summarized on Table 4.

Wells MW-1 through MW-7 and MW-13 and MW-14 are completed in clay or lime-like material and have lower hydraulic conductivities. As illustrated in Figure 8, the groundwater hydraulic gradient is steeper in the area of these wells. Wells MW-8 through MW-12 and MW-15 through MW-22 are completed in sandy soil, have higher hydraulic conductivity, and have a flatter hydraulic gradient. For example, south of the former shoreline in the clay soil, the average hydraulic gradient was approximately 0.032; and north of the former shoreline in the sandy soil, the average hydraulic gradient was approximately 0.009.

The thickness of the fill and sand aquifer resembles a "wedge" shape that increases in thickness towards Superior Bay. The thickness of the aquifer is measured from the water table down to the clay layer. The thickness ranges from 5 feet in MW-7 near the former shoreline to 14 feet thick north of the railroad tracks in MW-11. The thickness of the aquifer is expected to increase towards the northeast on the WWTP property; however, the clay layer was not encountered in the borings completed on the WWTP property.

1.8 Potential Receptors

1.8.1 Surface Waters

The site is located near an industrial boat slip on Superior Bay in Lake Superior. Stormwater runoff and groundwater from the site may enter the boat slip. A storm sewer conveys runoff from the city to the boat slip.

1.8.2 Groundwater Supply Wells

ENSR obtained well construction reports from the Wisconsin Geological and Natural History Survey (WGNHS) for the period of 1936 through the present for wells within 1.5 miles of the Site. These records were reviewed for potential receptors within 1,200-feet of the site, per NR 716. Copies of the well construction records obtained from WGNHS are included in **Appendix A**. Based on a review of the well construction records, there are no wells within 1,200-feet of the site. Municipal drinking water supply is obtained from Lake Superior via horizontal wells installed over a mile from the site in the bed of the lake.

1.8.3 Underground Utilities

A storm sewer line runs along the boundary of the former MGP property and Lakehead Cement and discharges into the boat slip in Superior Bay. Sanitary sewer lines are located along the railroad tracks, and crossing Lakehead Cement, to enter the WWTP property. Lakehead Cement has laterals for natural gas and water from main lines under East 1st Street. A water line for a fire hydrant is located near the former Horton Sphere. The remaining utilities are located either above ground or in the nearby road right-of-way. Based on the contaminant distribution found at the site, it does not appear that the utilities are acting as preferential flow pathways. Based on the results of the Sediment Investigation (March 2004), the sediment within the nearby storm sewer and sediment located at the storm sewer discharge point have not been measurably impacted by the site contaminants.

1.8.4 Underground and Aboveground Structures

Other than remnants of building foundations and gas holder bases, there are no known underground structures on the former MGP property or on the adjacent CLM, Lakehead Concrete, or WWTP properties.

The only aboveground structure on the former MGP property is the brick building which is currently used for storage by CLM. There are aboveground structures on the neighboring properties including a single family residence to the southwest, two Lakehead Concrete buildings to the southeast, numerous WWTP buildings to the east, and CLM buildings to the north.

1.9 Waste Characterization of Impacted Soil

Effective March 13, 2002, the United States Environmental Protection Agency (USEPA) vacated the ruling that provides for the use of the toxicity characteristic leaching procedure (TCLP) for determining whether MGP waste exhibits the characteristic of toxicity under the Resource Conservation and Recovery Act (RCRA; 40 CFR Part 261). Based on this ruling there is no current mechanism for MGP wastes to be regulated under RCRA. However, to determine soil disposal options and costs prior to remediation, ENSR will collect soil samples from the source area near the clay pipe for analysis of TCLP Metals, TCLP VOC, VOC, PAH, and Reactive Cyanide.

2.0 Remedial Action Options

2.1 Remedial Objectives

The overall objective for the site is to reduce site risks by removing or treating persistent MGP-related chemicals (tarry residues) in source soils. By removing or lessening the source soil concentrations, contaminants dissolved in the groundwater will be more susceptible to natural attenuation and concentration reduction. Implementing a remedial action should accomplish the following:

- Reduce human health risk from direct contact with contaminated soil;
- Reduce human health risk from contaminants which may volatilize into indoor and ambient air;
- Reduce the persistent groundwater contaminant source; and
- Address dissolved contaminants in groundwater.

An appropriate range of remedial alternatives is presented in the following sections. Each alternative identified has been evaluated in accordance with the evaluation criteria listed in NR 722.07 of the WAC.

2.2 Remedial Alternatives for Source Soil

2.2.1 Alternative #1 – Excavation and Off-Site Disposal of Source Soil

<u>Physical/Operations Description</u>: A portion of the source soils and tarry residues is relatively shallow and accessible. This alternative involves excavating accessible source soil and hauling it off-site for disposal at a WDNR approved solid waste recycling and disposal facility (RDF). Source soil contains tarry residues and is located along and at the terminus of the clay pipe found in trenches TR-9 and TR-10.

As shown in **Figure 9**, the amount of soil to be excavated is estimated at 2,000 cubic yards or approximately 2,800 tons. The actual amount excavated will depend on conditions encountered in the field (e.g., water table conditions, the amount of clean soil overlying the source soils, etc.). Clean fill material will be imported to the site to replace the excavated soils. The excavation will be done in cold weather to minimize odors and VOC emissions. A combination of fugitive air emission controls, air monitoring, or other measures are likely to be necessary to perform this alternative to protect public health. Other activities associated with this alternative are design and specifications development, subcontractor bidding and coordination, permitting, backfill compaction, soil sampling, final site restoration, reporting, and closure documentation.

<u>Technical Feasibility:</u> This alternative is considered technically feasible and would be effective in achieving the remedial objectives and goal of risk reduction. This alternative will immediately reduce the levels of soil contamination at the site and will reduce a source of contaminants to the groundwater. Excavation would not be feasible to address source soil impacts in areas that are not accessible, such as the tarry residues present along the railroad tracks.

<u>Economic Feasibility:</u> Excavation and off-site disposal of impacted soil is often considered a low to medium cost alternative at sites where active remediation is required. Therefore, this alternative is considered an economically feasible option to address the source soils described above. Considering all required work associated with implementing this alternative (from design through remedial action completion), the estimated cost for this alternative ranges from \$250,000 to \$350,000.

<u>Summary</u>: This alternative will accomplish risk reduction objectives for the impacted soils at the site with a high degree of success. This alternative will help reduce a source of contaminants to the groundwater. In addition, environmental benefits will be realized in a timely manner making this alternative a feasible remedial option for the Site.

2.2.2 Alternative #2 – In-Situ Chemical Oxidation

<u>Physical/Operations Description</u>: The portion of source soil and tarry residues that is not accessible for excavation may be treated using in-situ chemical oxidation (ISCO). The ISCO alternative involves injecting chemicals into the subsurface which will oxidize VOC and PAH contamination; thus converting the contaminants into innocuous compounds. ISCO utilizes chemical reactions, such as Fenton's chemistry, to create free radicals in the subsurface which react with organic compounds. The chemicals are introduced into the subsurface by injection through permanent injection wells, by injection through temporary direct push borings, or by placing them into an excavation prior to backfilling. The number and frequency of application points, along with the depths of the injections in the impacted soil, are determined based on contaminant concentrations and soil types. A typical time frame is weeks to months for the chemical reaction from one application to be completed.

ISCO is generally effective on contaminants dissolved in the groundwater and is less effective for NAPL or contamination adhered to soil. Bench scale and pilot scale treatability tests will be needed to evaluate ISCO performance on the tarry residues at the site. Surfactants, heat, and/or bubbles released from the chemical reaction can help to move contaminants from soils into groundwater where ISCO is more effective. Depending on the ISCO technology and chemicals selected, one to five rounds of treatment may be necessary to reduce NAPL and soil PAH and VOC concentrations.

<u>Technical Feasibility:</u> This alternative has been used at other MGP sites and is considered technically feasible. The tarry residues in the source soil at the site may respond to multiple ISCO applications. High groundwater pH at the site may limit the effectiveness of some reagents. Other limitations include the organic content of the soil at the site (wood chip and saw dust) which will consume some of the oxidant. This alternative would not be appropriate to treat the tarry material located along and at the terminus of the clay pipe due to the large volume of oxidant which would be required in that area. Bench scale tests need to be conducted to determine the safest and most effective oxidant to use at the site, and to develop cost estimates for pilot and full-scale remediation. Based on the bench scale test results, a pilot test may be conducted at the site to confirm the effectiveness of the selected chemistry. If the results are successful, then a full scale injection program may be initiated.

<u>Economic Feasibility</u>: This alternative may be economically feasible to treat source soil inaccessible to excavation depending on the number of injection rounds needed. Cost estimates will be determined based on the results of bench and pilot scale testing.

<u>Summary:</u> This alternative requires further bench and pilot scale studies to determine reagents, costs, and effectiveness. This alternative can help reduce the source of contaminants to the groundwater. This alternative provides flexibility to inject in specific areas where treatment is needed the most, and can be conducted in conjunction with soil excavation. In addition, environmental benefits will be realized in a timely manner making this alternative a feasible remedial option for the Site.

2.2.3 Alternative #3 – In Situ Thermal Treatment of Source Soil

<u>Physical/Operations Description</u>: This alternative involves a process where chemicals in the soil are volatilized by applying an electrical resistance heating system to the area of impacted soil and concurrently recovering the vapors by soil vapor extraction (SVE) methods. The chemicals recovered by the SVE system may require treatment by catalytic oxidation or carbon adsorption prior to discharging the air stream to the atmosphere. This method includes the installation of vertical and/or horizontal piping systems (by vertical and/or horizontal

drilling methods with some near surface excavation). Thermal treatment would boil away ground water in saturated deposits and also heat the vadose zone. The energy required depends in part on how fast groundwater re-enters the treatment zone. A temporary building would be required to shelter the SVE equipment for continuous operation and the site would have to be secured to avoid accidental contact with electrical equipment by the public.

Other activities associated with this alternative are design and specifications development, subcontractor bidding and coordination, permitting, operation and maintenance, soil verification sampling, system decommissioning, final site restoration, reporting, and closure documentation.

<u>Technical Feasibility:</u> This alternative would be capable of addressing source soils that are not accessible using *ex-situ* methods, such as excavation. The thermal treatment equipment and accompanying SVE system would require numerous above-ground utilities which would be difficult to protect in some site areas due to road and rail traffic. In addition, this alternative would be most effect on VOCs, and would not be effective on PAH compounds due to their high boiling points.

<u>Economic Feasibility</u>: Installation and operation of this energy-intensive system is typically a high cost alternative. Anticipated costs are roughly three to five times higher than excavation and removal on a unit cost basis. The widespread distribution of source soils in thin layers would not optimize the effect of heating.

<u>Summary:</u> This alternative will accomplish the treatment and risk reduction objectives for VOCs in the source soil, but would not be effective for PAH contamination. The technical challenges and the high capital and O&M costs to achieve the remediation goals do not make this alternative an attractive remedial option for the site.

2.2.4 Alternative #4 – Engineered Barrier Enhancements

<u>Physical/Operations Description</u>: This alternative involves installing an engineered concrete or asphalt cover over all areas with contaminant concentrations that pose a risk to human health. The cap would protect human health by eliminating pathways for direct contact with impacted soil and groundwater. The cap would also prevent infiltration and percolation of surface water through the soil and prevent the continued transport of contaminants into the groundwater. The cap may also reduce volatilization of contaminants to ambient air. The placement of the cap would require a surface that is a minimum of 1.5 inches thick, with greater thickness in areas of heavier and more frequent traffic. Current and future property owner(s) would be required to maintain the integrity of the surface on a regular basis, in accordance with a maintenance plan (developed by the property owner and/or consultant). Currently, the source area is covered by grass or gravel.

<u>Technical Feasibility:</u> Use of an engineered cap would reduce future infiltration and percolation of surface waters through the source area; however, it will not greatly reduce the current impacts to the groundwater. Soil contaminants currently are in direct contact with the groundwater. Therefore, without active soil remediation, the impacted soil will continue to serve as a source of groundwater contamination. In addition, the engineered cap will not reduce the volatilization to indoor air exposure pathway. Any utility construction to service new development on the property would require proper management of impacted soil. Consequently, maintaining the residually impacted soil at the Site may not significantly reduce future long-term liability.

<u>Economic Feasibility</u>: The cost to implement the placement and maintenance of an engineered cap are relatively low. However, future management of soil may add significant long-term liability and cost to the current and/or future owner(s).

<u>Summary:</u> Although this is a low cost method, there would be minimal reduction to future groundwater contamination, it would not reduce exposure to the volatilization to indoor air pathway, and long-term liability for the current responsible parties would not be significantly reduced. The engineered cap would address the direct contact exposure pathway and the help reduce the volatilization to ambient air pathway. The engineered

cap may be useful in conjunction with other technologies used to treat source soils, but would have limited value as a stand-alone remedy at this site.

2.2.5 Alternative #5 – No Action

The no action alternative would include leaving the source soils in place. However, it is unknown if the contaminants would eventually degrade to acceptable levels. This alternative involves no costs for addressing the problem and has no limitations for implementation. Although technically and economically feasible, this alternative does not eliminate or reduce the existing risks and will not lead to site closure under WDNR rules.

3.0 Selected Remedial Action

The overall objective for implementing a remedial strategy at the Site is to realize a significant risk reduction benefit by removing or treating soil containing tarry residues that are a long-term source of groundwater contaminants. Without the continuing contribution of contaminants from source soils, monitored natural attenuation will effectively remediate groundwater.

Based on the evaluation of the various remedial alternatives presented, the following remedial actions have been selected:

- 1. Excavation and disposal of source soil located along and at the terminus of the clay pipe.
- 2. ISCO to treat source soils, as necessary.
- 3. Monitored natural attenuation to reduce dissolved concentrations in the groundwater.

The source soil removal will eliminate the direct contact risk and will aid in improving groundwater quality in the immediate area and downgradient of the excavation. The excavation will reduce human health risk immediately and is economically feasible. Soil samples will be collected for waste characterization prior to disposal. Excavation is not feasible along the railroad tracks or where thin source soil layers are deep below the ground surface. The estimated cost of excavation is \$250,000 to \$350,000.

After source soil removal, tarry residues that are inaccessible to excavation will be evaluated for treatment using ISCO. ISCO of the tarry residue as identified by soil sampling and the TarGOST survey could reduce the source of contaminants and could reduce the dissolved VOC and PAH concentrations in groundwater. Potentially, ISCO could reduce risk after one or more applications of a selected oxidant. Design and costs for the ISCO remedy will require bench and pilot scale treatability testing. Soil samples will be collected during the soil excavation in the source area. Bench scale study will be conducted on the soil samples to determine the most feasible and effective oxidant and oxidant demand. Based on the results of the bench scale study, an oxidant may be selected and a pilot study may be completed at the site. If the results of the pilot study indicate that the oxidant performed well, then a larger scale ISCO may be initiated, to the extent that it feasibly meets overall remedial objectives. More than one injection of reagent may be needed in some areas to achieve the remedial goals. ENSR has identified the following candidates for the bench scale study:

- Cool-Ox[™], a proprietary blend by Deep-Earth Technology. According to Deep-Earth Technology, Cool-Ox generates hydrogen peroxide from solid peroxygens that are injected into the soil or groundwater in an aqueous suspension. Once in place, the peroxygens react with water to produce hydrogen peroxide. Metal catalysts are not needed for the reaction to occur, the reaction can occur in alkaline environments, and heat is not generated during the reaction.
- VeruTec Technologies, Inc. combines surfactant and oxidant chemistries using a controlled dissolution and desorption process (by dilute surfactant mixtures) with concomitant chemical destruction processes. VeruSOL[®] is a proprietary surfactant specifically designed for MGP byproducts which is combined with traditional oxidant chemistries, such as Fenton's.
- Geo-Cleanse[®] International, Inc. utilizes sodium persulfate for chemical oxidation, which is effective at high pH environments. Also, the sulfate from the persulfate can be utilized during aerobic biodegradation of the residual contaminants after treatment.
- In-Situ Oxidative Technologies, Inc (Isotec) utilizes liquid hydrogen peroxide and a site-specific patented chelated iron catalyst mixed to the same pH as the groundwater. ISOTEC injects stabilized 12% hydrogen peroxide followed by the chelated iron catalyst into the subsurface.

After source soil removal/treatment, monitored natural attenuation will be the final stage of the remedy for groundwater. Natural attenuation is defined by the US Environmental Protection Agency as "the biodegradation, dispersion, dilution, sorption, volatilization, and/or chemical and biochemical stabilization of contaminants to effectively reduce contaminant toxicity, mobility, or volumes to levels that are protective of human health and the ecosystem" (Brady, et al., 1997). Contaminants present in soil and groundwater will attenuate via naturally occurring biotic and abiotic processes. Natural attenuation processes and rates of contaminant degradation will be monitored by changes in contaminant concentration versus time and hydrogeochemical parameters of the affected aquifer.

Patterns consistent with natural attenuation have been observed on-site by the detection of low dissolved oxygen and ORP in monitoring wells located within the source area thereby indicating that natural biodegradation is taking place. The presence of petroleum-related compounds (site fingerprinting analytical results indicated an unresolved complex petroleum mixture) will provide bioavailable carbon to assist in the natural attenuation process.

Remediation by natural attenuation for the groundwater will be considered successful if the contaminant plume is stable or receding and VOC/PAH and geochemical indicator data provide evidence that natural attenuation is occurring at a rate sufficient to protect human health and the environment. Post-remediation groundwater monitoring will be conducted quarterly for two years to determine concentration trends and to verify that conditions conducive to natural attenuation are present at the site. Upon the successful demonstration of monitored natural attenuation, site closure documentation will be prepared for submittal to WDNR. The estimated cost for monitored natural attenuation and site closure documentation is \$150,000 to \$200,000.

Superior Water Light & Power Former MGP Site East 1st Street and Water Street Superior, Wisconsin

Remedial Action Options Report WDNR BRRTS #02-16-275446

Certification - Professional Geologist/Hydrogeologist

I, William M. Gregg, hereby certify that I am a hydrogeologist as that term is defined in s. NR 712.03 (1), Wis. Adm. Code and a registered professional geologist in the State of Wisconsin, registered in accordance with the requirements of ch. GHSS 2, Wis. Adm. Code; that this document has been prepared in accordance with the Rules of Professional Conduct, per ch. GHSS 5 of the Wis. Adm. Code; and that, to the best of my knowledge, all information contained in this document is correct and the document was prepared in compliance with all applicable requirements in chs. NR 700 to 726, Wis. Adm. Code.

William M. Streen Signature

2/5/08 Date

Seal

Superior Water Light & Power Former MGP Site

East 1st Street and Water Street Superior, Wisconsin

Remedial Action Options Report

WDNR BRRTS #02-16-275446

Certification - Professional Engineer

I, Scott Tarmann, hereby certify that I am a registered professional engineer in the State of Wisconsin, registered in accordance with the requirements of ch. A-E 4, Wis. Adm. Code; that this document has been prepared in accordance with the Rules of Professional Conduct in ch. A-E 8, Wis. Adm. Code; and that, to the best of my knowledge, all information contained in this document is correct and the document was prepared in compliance with all applicable requirements in chs. NR 700 to 726, Wis. Adm. Code.



1/31/ '08

Date

Figures



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	SWL&P Property Boundary
B-12	Geoprobe Soil Boring
- MW-7	Monitoring Well
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		RAOR	SWL&P FORMER MGP		SUPERIOR, WISCONSIN SUPERIOR, MERCONSIN		SCALE: DATE: PROJECT NUMBER: FAA. (332) 324-1		1 = 75 10/1/10/ 03413-038

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Tables

Table 1Summary of Surface Soil ResultsSuperior Water, Light and Power Former MGPSuperior, Wisconsin

Sample ID	WDNR Soil	B-15-1-3	MW-13	MW-17	MW-20	SS-1	SS-2	SS-3	SS-4	SS-5	SS-6	SS-7	SS-8	SS-8 DUP	SS-9	SS-10	SS-11	SS-12	SS-13
Sample Depth	Standards ^a	1-3'	3-4'	2-3'	2-3'	0-2'	0-2'	0-2'	0-2'	0-2'	0-2'	0-2'	0-2'	0-2'	0-2'	0-2'	0-2'	0-2'	0-2'
VOC																			
Benzene	1,100	21,000	<32	<50	280	310	68	260	<25	5,600	1,000	150	80	78	<25	2,300	<25	<26	310
Chloromethane	NE °					<25	<25	<25	<25	130	<25	<25	<25	<26	<25	<26	<25	<26	<25
Ethylbenzene	NE	5,100	<32	<50	48	<25	41	<25	<25	150	220	130	<25	39	<25	210	<25	<26	350
Isopropylbenzene	NE	160				<25	<25	<25	<25	98	<25	<25	<25	<26	<25	<26	<25	<26	<25
N-Butylbenzene	NE					<25	<25	<25	<25	53	<25	<25	<25	<26	<25	<26	<25	<26	<25
n-Propylbenzene	NE					<25	<25	<25	<25	87	<25	<25	<25	<26	<25	<26	<25	<26	<25
p-Isopropyltoluene	NE		<32	<50	<45	<25	<25	<25	<25	42	<25	<25	<25	<26	<25	<26	<25	<26	<25
sec-Butylbenzene	NE					<25	<25	<25	<25	37	<25	<25	<25	<26	<25	<26	<25	<26	<25
Styrene	NE	<50				<25	<25	<25	<25	<25	<25	<25	<25	<26	<25	47	<25	<26	<25
Toluene	NE	9,300	<32	<50	230	430	210	240	<25	3,100	920	440	97	88	<25	2,800	<25	38	390
1,2,4-Trimethylbenzene	NE	980	^b			<25	130	51	<25	390	56	150	<25	<26	<25	160	<25	<26	100
1,3,5-Trimethylbenzene	NE	460				<25	62	<25	<25	100	<25	99	<25	<26	<25	89	<25	<26	56
Xylene, o	NE	1,400	<32	<50	63	<25	170	71	<25	680	60	190	<25	<26	<25	330	<25	<26	150
Xylenes, m + p	NE	5,800	<64	<100	170	120	270	110	<50	1,100	330	510	<50	<51	<50	1,300	<50	<26	330
PAH																			
Acenaphthene	60,000,000	95Q	<3.9	<3.1	86	13	2,100	4.7	4.3	35	2,100	360	120	52	<3.5	400	<3.9	9.7	170
Acenaphthylene	360,000	920	<3.8	<3.0	41	<3.9	1,300	27	11	330	7,800	3,200	600	210	5	1,700	<3.8	210	4,800
Anthracene	30,000,000	350	<4.7	<3.8	220	<4.9	5,300	23	14	480	23,000	2,000	670	170	<4.2	1,100	10	68	1,100
Benzo(a)anthracene	3,900	1,600	10	7.4	410	<7.3	9,500	57	31	2,000	28,000	5,100	1,700	450	8.8	1,100	25	140	1,400
Benzo(a)pyrene	390	1,900	10	9.5	410	<3.9	9,100	55	24	2,300	23,000	4,500	1,800	630	10	2,500	26	280	4,800
Benzo(b)fluoranthene	390	1,700	7.9	7.5	320	<3.8	7,700	52	22	2,000	17,000	3,100	1,100	400	7.7	1,300	24	140	2,100
Benzo(ghi)perylene	39,000	1,900	6.9	7.3	130	<4.9	3,000	56	26	1,100	8,300	2,100	1,100	400	11	1,600	17	210	3,100
Benzo(k)fluoranthene	39,000	1,800	8.5	7.6	340	<4.2	8,100	48	28	1,800	20,000	4,200	1,400	440	7.8	1,800	20	150	2,500
Chrysene	390,000	2,200	11	8.8	410	<6.0	11,000	93	52	2,100	34,000	6,900	1,900	540	13	1,600	31	170	1,800
Dibenz(a,h)anthracene	390	570	<3.6	<2.9	41	<3.8	1,400	16	<3.8	390	2,700	720	260	110	<3.2	460	<3.6	60	920
Fluoranthene	40,000,000	1,600	20	11	950	<3.9	22,000	64	67	3,100	20,000	9,300	3,400	590	12	1,600	39	170	1,600
Fluorene	40,000,000	110	<4.5	<3.6	85	<4.7	2,400	12	<4.7	60	650	310	45	16	<4	99	<4.5	17	160
Indeno(1,2,3-cd)pyrene	3,900	1,400	5	5.8	130	<3.4	3,100	35	18	1,000	6,800	1,600	760	290	7.3	1,100	13	150	2,200
1-Methylnaphthalene	70,000,000	860	<4.0	11	65	14	630	320	14	660	2,300	390	170	92	5.7	470	12	45	480
2-Methylnaphthalene	40,000,000	1,100	<4.1	14	79	9.1	910	420	25	820	5,800	740	350	170	8.7	910	19	67	750
Naphthalene ^d	110,000		<32	<50	200	<25	460	100	<25	940	3,200	1,800	500	290	<25	3,100	90	210	910
Naphthalene	110,000	840	<5.3	9.8	160	14	1,700	260	27	560	17,000	1,700	750	310	13	1,800	13	100	2,100
Phenanthrene	390,000	1,200	12	12	1,100	4.8	22,000	200	100	1,500	6,900	4,100	2,400	450	14	1,500	34	150	1,400
Pyrene	30,000,000	2,900	19	11	1,100	3.4	24,000	88	97	3,200	72,000	16,000	5,000	1,000	19	3,000	48	270	2,900

Notes:

Results are reported in parts per billion, or micrograms per kilogram (ug/kg).

Bold results indicate value exceeds Direct Contact RCL.

Only shallow soil samples (less than 4-feet deep) are reported in this table.

(a) Protection of Human Health from Direct Contact with Contaminated Soil (top 4 feet of soil), Table 2, NR 746 (VOCs only).

Soil Cleanup Levels of Polycyclic Aromatic Hydrocarbons Interim Guidance, WDNR Publication RR-519-97 (PAH only).

(b) -- indicates sample was not analyzed for this parameter.

(c) NE - Not Established.

(d) Analysis for naphthalene was performed using two analytical methods (8260 and 8270).

Table 1Summary of Surface Soil ResultsSuperior Water, Light and Power Former MGPSuperior, Wisconsin

Sample ID	WDNR Soil	SS-14	SS-15	SS-16	SS-17	SS-18	SS-19	SS-20	T1-S1	T1-S2	T2-S1	T7-S1	T8-S1	T9-S2	T9-S2 DUP
Sample Depth	Standards *	0-2'	0-2'	0-2'	0-2'	0-2'	0-2'	0-2'	1.5'	2-2.5'	2.5-3'	3-3.5'	3-4'	2'	2'
VOC															
Benzene	1,100	2,000	39	<25	<25	<25	<26	33	390	<25	<25	<25	240,000	400	210
Chloromethane	NE ^c	<25	<25	<25	<25	<25	<26	<25							
Ethylbenzene	NE	92	<25	<25	<25	<25	<26	<25	170	<25	<25	<25	<1,000	<25	<25
Isopropylbenzene	NE	<25	<25	<25	<25	<25	<26	<25							
N-Butylbenzene	NE	<25	<25	<25	<25	<25	<26	<25							
n-Propylbenzene	NE	<25	<25	<25	<25	<25	<26	<25							
p-Isopropyltoluene	NE	<25	<25	<25	<25	<25	<26	<25							
sec-Butylbenzene	NE	<25	<25	<25	<25	<25	<26	<25							
Styrene	NE	<25	<25	<25	<25	<25	<26	<25							
Toluene	NE	1,100	39	<25	<25	31	60	71	540	<25	<25	<25	150,000	650	270
1,2,4-Trimethylbenzene	NE	160	<25	<25	<25	<25	<26	33							
1,3,5-Trimethylbenzene	NE	160	<25	<25	<25	<25	<26	<25							
Xylene, o	NE	640	<25	<25	<25	<25	<26	50	340	<25	<25	<25	9,500	81	63
Xylenes, m + p	NE	1,600	<50	<50	<50	<50	<51	92	620	<25	<25	<25	18,000	240	180
PAH		, i													
Acenaphthene	60,000,000	4.1	<4.3	<3.9	<4.1	16	27	<3.8	<280	<22	<21	<22	<22	<21	76
Acenaphthylene	360,000	19	<4.2	<3.8	<4	10	450	16	560	<16	<16	<16	<17	49	56
Anthracene	30,000,000	15	<5.1	<4.7	<4.9	39	550	23	<200	<16	<16	18	<16	35	210
Benzo(a)anthracene	3,900	42	20	<7	8.9	76	1,600	73	290	<18	<17	<18	<18	320	650
Benzo(a)pyrene	390	55	19	<3.8	8.6	77	1,600	78	250	<16	<16	<16	<17	680	900
Benzo(b)fluoranthene	390	36	15	3.9	7.3	57	1,400	64	400	<14	<14	<14	<15	470	700
Benzo(ghi)perylene	39,000	39	9.6	<4.7	5.4	36	500	45	420	<15	<15	<15	<15	540	620
Benzo(k)fluoranthene	39,000	43	18	<4	8.7	68	1,600	64	290	<17	<17	<17	<17	470	660
Chrysene	390,000	54	22	6.5	11	85	1,500	82	510	<18	<17	<18	<18	400	680
Dibenz(a,h)anthracene	390	8.2	<4	<3.6	<3.8	12	250	13	<190	<14	<14	<14	<15	160	200
Fluoranthene	40,000,000	59	35	8.7	16	180	3,200	130	610	<14	14	21	<14	420	1100
Fluorene	40,000,000	4.8	<4.9	<4.5	<4.7	16	35	6	<210	<16	<16	<16	<17	<16	80
Indeno(1,2,3-cd)pyrene	3,900	29	9	<3.3	4.5	33	510	39	430	<15	<15	<15	<15	490	610
1-Methylnaphthalene	70,000,000	68	4.8	5.2	4.5	33	74	39	2200	<18	<18	28	<19	<18	27
2-Methylnaphthalene	40,000,000	100	6.3	5.2	6.4	47	110	61	4,700	<16	<16	45	<16	25	44
Naphthalene ^d	110,000	130	<25	49	<25	82	160	180							
Naphthalene	110,000	90	5.9	<5.3	6.5	51	150	88	15,000	<22	<22	130	58	55	91
Phenanthrene	390,000	55	15	6.1	11	160	760	76	870	<14	<14	54	<15	210	830
Pyrene	30,000,000	74	38	8.3	18	180	2,800	150	490	<16	<16	28	<16	480	1,000

Table 2aSummary of Deep Phase II Soil Analytical ResultsSWLP MGPSuperior, Wisconsin

Analyte	Units	WDNR Soil Standards ^a	T3-S1	T4-S1	T6-S1	T7-S2	T9-S1	B-1-8-10	B-2-8-10	B-3-6-8	B-3-20-22	B-4-8-10	B-5-8-10	B-5-8-10-DUP	B-6-8-10	B-7-8-10
Metals			8 Ft	6.5-7 Ft	8 Ft	5-6 Ft	5 Ft	8-10 Ft	8-10 Ft	6-8 Ft	20-22	8-10 Ft	8-10 Ft	8-10 Ft	8-10 Ft	8-10 Ft
Arsenic	mg/Kg		3.1	4.6	3.3	2.8	2.9	3.4	2.9	2.7	3.0	3.4	3.9	2.9	3.6	2.6
Barium	mg/Kg		62	88	170	200	140	250	210	160	120	170	140	100	220	150
Cadmium	mg/Kg		0.16	0.26	0.26	0.25	0.22	0.29	0.25	0.22	0.26	0.33	0.90	0.25	0.30	0.24
Chromium	mg/Kg		6.8	7.5	38	39	35	50	43	40	40	45	36	43	46	43
Lead	mg/Kg		7.3	32	19	8.4	8.4	12	8.9	7.4	10	10	43	8.4	11	7.8
Selenium	mg/Kg		1.9	2.2	0.41	0.46	0.33	0.38	0.62	0.53	0.36	0.50	0.29	0.47	0.66	0.45
Silver	mg/Kg		<0.27	<0.23	<0.20	<0.19	<0.19	<0.21	<0.20	<0.20	<0.21	<0.21	<0.19	<0.20	<0.21	<0.19
Mercury	mg/Kg		<0.0094	<0.0078	0.056	<0.0067	0.018	<0.0073	<0.0068	<0.0068	<0.0072	<0.0071	<0.0067	<0.0067	<0.0072	<0.0067
Cyanide, total	mg/kg		<0.44	<0.37	<0.32	<0.32	<0.31	<0.34	<0.32	<0.32	<0.34	<0.34	<0.32	<0.32	<0.34	< 0.32
РАН																
1-Methylnaphthalene	ug/kg	23,000	<25	180	99,000	<18	44	<20	<18	35,000	<20	<19	<18	<18	<20	<18
2-Methylnaphthalene	ug/kg	20,000	<22	260	150,000	<15	82	<17	<16	56,000	<17	<17	<16	16	<17	<15
Acenaphthene	ug/kg	38,000	<30	<25	77,000	<21	<21	<23	<22	2,400	<23	<23	<21	<21	<23	<21
Acenaphthylene	ug/kg	700	<23	<19	31,000	<16	27	<18	<16	9,900	<17	<17	<16	<16	<17	<16
Anthracene	ug/kg	3,000,000	<22	35	65,000	<15	23	<17	<16	15,000	<17	<17	<16	<16	<17	<15
Benzo(a)anthracene	ug/kg	17,000	<24	57	36,000	<17	21	<19	<18	9,600	<19	<19	<17	<18	<19	<17
Benzo(a)pyrene	ug/kg	48,000	<23	36	28,000	<16	19	<18	<16	8,200	<17	<17	<16	<16	<17	<16
Benzo(b)fluoranthene	ug/kg	360,000	<20	36	16,000	<14	<14	<15	<14	3,200	<15	<15	<14	<14	<15	<14
Benzo(g,h,i)perylene	ug/kg	6,800,000	<21	24	14,000	<15	<15	<16	<15	3,300	<16	<16	<15	<15	<16	<15
Benzo(k)fluoranthene	ug/kg	870,000	<24	32	19,000	<17	20	<18	<17	5,300	<18	<18	<17	<17	<18	<17
Chrysene	ug/kg	37,000	<24	59	36,000	<17	23	<19	<18	8,500	<19	<19	<17	<18	<19	<17
Dibenzo(a,h)anthracene	ug/kg	38,000	<20	<17	4,200	<14	<14	<15	<14	950	<15	<15	<14	<14	<15	<14
Fluoranthene	ug/kg	500,000	<19	170	63,000	<14	36	<15	<14	17,000	<15	<14	<14	<14	<15	<14
Fluorene	ug/kg	100,000	<23	<19	48,000	<16	16	<18	<15	13,000	<17	<17	<16	<16	<17	<16
Indeno(1,2,3-cd)pyrene	ug/kg	680,000	<21	19	13,000	<15	<15	<16	<16	3,300	<16	<16	<15	<15	<16	<15
Naphthalene	ug/kg	400	<31	170	66,000	<22	110	<24	<22	190,000	<24	<23	<22	25	<24	57
Phenanthrene	ug/kg	1,800	<20	200	170,000	<14	91	<15	<14	50,000	<15	<15	<14	17	<15	<14
Pyrene	ug/kg	8,700,000	<22	120	88,000	<15	41	<17	<16	23,000	<17	<17	<16	<16	<17	<15
BTEX																
Benzene	ug/kg	5.5	<25	<25	21,000	<25	250	<25	<25	42,000	1,500	<25	<25	<25	4,000	67
Ethylbenzene	ug/kg	2,900	<25	<25	160,000	<25	220	<25	<25	13,000	<25	<25	<25	<25	<25	<25
Toluene	ug/kg	1,500	<25	41	7,300	<25	2,200	<25	<25	76,000	75	<25	<25	<25	<25	<25
Xylene, -o	ug/kg	4,100 ^b	<25	<25	58,000	<25	3,000	<25	<25	37,000	<25	<25	<25	<25	<25	<25
Xylenes, -m, -p	ug/kg	4,100 ^b	<25	42	120,000	<25	6,400	<25	<25	73,000	<25	<25	<25	<25	<25	<25
рН																
pH, Laboratory		NA	12.6	12.0	7.5	7.82	8.1	8.06	7.85	8.03	8.40	7.96	8.13	8.14	8.19	7.96

a. Residual Contaminant Levels (RCL) for Protection of Groundwater. No RCLs established for metals. RCLs for PAH are from Table 1, Soil Cleanup Levels for PAHs Interim Guidance. RCL for BTEX are from Table 1, WAC NR 720.

b. The RCL for xylene is 4,100 ug/kg for the sum of all xylenes.

Note: Bold results indicate concentrations greater than applicable RCL.

Approximate sample depth beneath sample ID is reported in feet below grade.

Only deep soil samples (greater than 4-feet deep) are reported in this table.

Samples were collected for the Phase II Investigation (see Phase II Report dated January 2002).

Table 2a Summary of Deep Phase II Soil Analytical Results SWLP MGP Superior, Wisconsin

Analyte	Units	WDNR Soil Standards ^a	MW-1-12-14	MW-2-12-14	MW-3-6-8	MW-4-8-10	MW-5-4-6	MW-6-8-10	MW-7-8-10	MW-7-8-10-DUP	MW-7-18-20
Metals			12-14 Ft	12-14 Ft	6-8 Ft	8-10 Ft	4-6 Ft	8-10 Ft	8-10 Ft	8-10 Ft	18-20 Ft
Arsenic	mg/Kg		3.2	2.9	2.7	3.3	6.2	2.5	4.5	7.7	3.4
Barium	mg/Kg		420	240	150	170	520	140	200	180	290
Cadmium	mg/Kg		0.29	0.24	0.21	0.28	0.64	0.22	0.39	0.44	0.28
Chromium	mg/Kg		43	39	35	40	15	28	9.7	11	41
Lead	mg/Kg		12	9.4	7.6	10.0	310	18	120	450	12
Selenium	mg/Kg		<0.30	0.41	0.34	0.54	2.1	<0.29	3.9	4.4	<0.29
Silver	mg/Kg		<0.22	<0.21	<0.19	<0.20	<0.23	<0.21	<0.24	<0.26	<0.22
Mercury	mg/Kg		0.018	0.014	0.0089	0.016	0.015	0.083	<0.0083	0.011	0.0094
Cyanide, total	mg/kg		<0.36	<0.34	<0.32	<0.34	<0.37	<0.34	<0.39	<0.43	<0.35
РАН											
1-Methylnaphthalene	ug/kg	23,000	<20	<19	2,100	<19	<21	200	1,000	2,400	<20
2-Methylnaphthalene	ug/kg	20,000	<17	<16	3,300	19	<18	290	1,700	3,400	<17
Acenaphthene	ug/kg	38,000	<24	<23	<210	<22	<25	640	750	1,500	<24
Acenaphthylene	ug/kg	700	<18	<17	390	<17	<19	620	1,000	1,900	<18
Anthracene	ug/kg	3,000,000	<17	<16	990	<16	<18	1,400	880	1,900	<17
Benzo(a)anthracene	ug/kg	17,000	<20	<18	810	<18	<21	3,300	1,300	2,700	<19
Benzo(a)pyrene	ug/kg	48,000	<18	<17	640	<17	<19	4,800	1,500	3,400	<18
Benzo(b)fluoranthene	ug/kg	360,000	<16	<15	300	<15	<17	1,800	880	1,900	<16
Benzo(g,h,i)perylene	ug/kg	6,800,000	<17	<16	280	<15	<18	3,700	1,100	3,000	<16
Benzo(k)fluoranthene	ug/kg	870,000	<19	<18	430	<17	<20	3,400	1,400	2,900	<19
Chrysene	ug/kg	37,000	<20	<18	740	<18	<21	4,200	1,900	4,200	<19
Dibenzo(a,h)anthracene	ug/kg	38,000	<16	<15	<140	<15	<17	650	<350	650	<16
Fluoranthene	ug/kg	500,000	<15	<14	1,200	<14	<16	9,000	2,700	6,000	<15
Fluorene	ug/kg	100,000	<18	<17	850	<17	<19	<180	660	1,400	<18
Indeno(1,2,3-cd)pyrene	ug/kg	680,000	<17	<16	250	<15	<18	3,300	1,000	2,700	<16
Naphthalene	ug/kg	400	<25	<23	11,000	<23	<26	1,000	20,000	41,000	<24
Phenanthrene	ug/kg	1,800	<16	<15	3,500	56	<17	2,600	2,600	5,900	<16
Pyrene	ug/kg	8,700,000	<17	<16	1,800	<16	<18	12,000	3,700	8,000	<17
BTEX											
Benzene	ug/kg	5.5	<25	<25	2,800	160,000	<25	100	1,100,000	910,000	640
Ethylbenzene	ug/kg	2,900	<25	<25	580	<500	<25	84	28,000	25,000	110
Toluene	ug/kg	1,500	<25	<25	2,200	36,000	<25	110	1,200,000	1,100,000	1,100
Xylene, -o	ug/kg	4,100 ^b	<25	<25	1,100	1,600	<25	49	270,000	260,000	270
Xylenes, -m, -p	ug/kg	4,100 ^b	<25	<25	2,300	5,700	<25	76	380,000	360,000	580
рН											
pH, Laboratory		NA	8.13	8.12	8.02	8.23	11.9	11.9	12.4	12.2	8.60

Table 2bSummary of Deep Phase II, Part II Soil Analytical ResultsSWLP MGPSuperior, Wisconsin

Analyte	Units	WDNR Soil Standard ^a	B-8-6-8	B-9-10-12	B-10-6-8	B-11-1-3	B-11-10-12	B-12-8-10	B-13-15-16	B-14-15-16	B-14-11-12	B-15-6-8	B-16-6-8	B-17-6-8	B-18-10-12
Metals															
Cyanide, total	mg/kg	NE ^b	^c								3				
PAH															
1-Methylnaphthalene	ug/kg	23,000				73	3,400	2,300	85	<20		270Q	25Q	730	<19
2-Methylnaphthalene	ug/kg	20,000				74	3,400	3,200	77	<17		420Q	32Q	1,300	<16
Acenaphthene	ug/kg	38,000				<23	3,500	820Q ^d	82	<23		<220	23Q	<210	<22
Acenaphthylene	ug/kg	700				25Q	930	630Q	<17	<18		<160	<17	310Q	<17
Anthracene	ug/kg	3,000,000				<17	2,300	420Q	<16	<17		180Q	<17	<160	<16
Benzo(a)anthracene	ug/kg	17,000				52Q	2,700	690Q	<18	<19		330Q	<19	410Q	<18
Benzo(a)pyrene	ug/kg	48,000				51Q	2,300	690Q	<17	<18		270Q	<17	180Q	<17
Benzo(b)fluoranthene	ug/kg	360,000				30Q	1,200	500Q	<15	<15		210Q	<15	280Q	<15
Benzo(g,h,i)perylene	ug/kg	6,800,000				54	1,900	1,000Q	<16	<16		300Q	<16	510	<15
Benzo(k)fluoranthene	ug/kg	870,000				43Q	1,800	580Q	<18	<18		260Q	<18	280Q	<17
Chrysene	ug/kg	37,000				57Q	3,400	1000Q	<18	<19		340Q	<19	480Q	<18
Dibenzo(a,h)anthracene	ug/kg	38,000				<16	460Q	<350	<15	<15		<140	<15	<140	<15
Fluoranthene	ug/kg	500,000				30Q	4,200	1,500	<14	<15		780	<14	910	<14
Fluorene	ug/kg	100,000				<18	2,100	<400	23Q	<18		190Q	<17	230Q	<17
Indeno(1,2,3-cd)pyrene	ug/kg	680,000				36Q	1,200	710Q	<16	<16		<150	<16	310Q	<15
Naphthalene	ug/kg	400				660	8,900	36,000	42Q	410		4,900	410	7,900	<23
Phenanthrene	ug/kg	1,800				32Q	9,200	2,200	42Q	<15		1,200	41Q	1,800	<15
Pyrene	ug/kg	8,700,000				98	8,100	2,000	<16	<17		1,400	20Q	1,200	<16
VOC															
1,2,4-Trimethylbenzene	ug/kg	NE				130	7,100	38,000	220	71		130,000	1,400	1,900	<25
1,3,5-Trimethylbenzene	ug/kg	NE				79	5,200	24,000	<100	<25		83,000	700	1,200	<25
Isopropylbenzene	ug/kg	NE				<25	<1000	<5000	<100	<25		<2,500	74	<50	<25
Styrene	ug/kg	NE				<25	40,000	140,000	<100	<25		240,000	290	1,400	<25
Benzene	ug/kg	5.5	54,000	100	120,000	2,200	240,000	590,000	27,000	12,000		76,000	10,000	16,000	<25
Ethylbenzene	ug/kg	2,900	380	<25	410	120	7,200	45,000	370	160		100,000	3,500	350	<25
Toluene	ug/kg	1,500	<130	<25	59,000	900	340,000	1,700,000	460	230		790,000	5,500	16,000	<25
Xylene, -o	ug/kg	4,100 ^e	<130	<25	1,700	160	35,000	150,000	<100	39Q		310,000	2,100	3,400	<25
Xylenes, -m, -p	ug/kg	4,100 ^e	1,100	<25	6,700	260	130,000	540,000	260	140		1,100,000	7,600	11,000	<25

a. Residual Contaminant Levels (RCL) for Protection of Groundwater. No RCLs established for metals. RCLs for PAH are from Table 1, Soil Cleanup Levels for PAHs Interim Guidance. RCL for VOCs are from Table 1, WAC NR 720.

b. NE = None established.

c. --- = Not analyzed.

d. Q means the analyte has been detected between the limit of detection and limit of quantification. The results are qualified as approximate concentrations due to the uncertainty of analyte concentrations within this range.

e. The RCL for xylene is 4,100 ug/kg for the sum of all xylenes.

Note: Bold results indicate concentrations greater than applicable RCL.

Approximate sample depth beneath sample ID is reported in feet below grade.

Only deep soil samples (greater than 4-feet deep) are reported in this table.

Samples were collected for the Phase II Part II Investigation (see Phase II, Part II Report dated February 2003).

Table 2bSummary of Deep Phase II, Part II Soil Analytical ResultsSWLP MGPSuperior, Wisconsin

Analyte	Units	WDNR Soil Standard ^a	B-19-10-12	B-20-10-12	B-21-10-12	B-21-10-12- dup	B-22-8-10
Metals		e la l'alla a					
Cyanide, total	mg/kg	NE ^b					
PAH							
1-Methylnaphthalene	ug/kg	23,000	<18	28Q	<18	<19	<19
2-Methylnaphthalene	ug/kg	20,000	<16	44Q	<15	<16	<17
Acenaphthene	ug/kg	38,000	<21	<20	<21	<22	<23
Acenaphthylene	ug/kg	700	<16	<15	<16	<17	<17
Anthracene	ug/kg	3,000,000	<16	23Q	<15	<16	<17
Benzo(a)anthracene	ug/kg	17,000	<18	<16	<17	<18	<19
Benzo(a)pyrene	ug/kg	48,000	<16	<15	<16	<17	<17
Benzo(b)fluoranthene	ug/kg	360,000	<14	<13	<14	<15	<15
Benzo(g,h,i)perylene	ug/kg	6,800,000	<15	<14	<15	<16	<16
Benzo(k)fluoranthene	ug/kg	870,000	<17	<16	<17	<18	<18
Chrysene	ug/kg	37,000	<18	<16	<17	<18	<19
Dibenzo(a,h)anthracene	ug/kg	38,000	<14	<13	<14	<15	<15
Fluoranthene	ug/kg	500,000	<14	<13	<14	<14	<14
Fluorene	ug/kg	100,000	<16	<15	<16	<17	<17
Indeno(1,2,3-cd)pyrene	ug/kg	680,000	<15	<14	<15	<16	<16
Naphthalene	ug/kg	400	<22	<20	<22	<23	<23
Phenanthrene	ug/kg	1,800	<14	22Q	<14	<15	<15
Pyrene	ug/kg	8,700,000	<16	<14	<15	<16	<17
VOC							
1,2,4-Trimethylbenzene	ug/kg	NE	<25	<25	<25	<25	<25
1,3,5-Trimethylbenzene	ug/kg	NE	<25	<25	<25	<25	<25
Isopropylbenzene	ug/kg	NE	<25	<25	<25	<25	<25
Styrene	ug/kg	NE	<25	<25	<25	<25	<25
Benzene	ug/kg	5.5	36Q	54Q	<25	<25	<25
Ethylbenzene	ug/kg	2,900	<25	<25	<25	<25	<25
Toluene	ug/kg	1,500	<25	35Q	<25	<25	<25
Xylene, -o	ug/kg	4,100 ^e	<25	<25	<25	<25	<25
Xylenes, -m, -p	ug/kg	4,100 ^e	<25	<25	<25	<25	<25

Table 2cSummary of Deep Phase II, Part III Soil Analytical ResultsSWLP MGPSuperior, Wisconsin

Ameluta	Line: to	WDNR Soil	B-24	B-25	B- 26	B-27	B-28	B-29	B-30	B-31	MW-8	MW-9	MW-10	MW-11	MW-11	MW-12
Analyte	Units	Standards ^a	5-6 Ft	7-8 Ft	6-7 Ft	7-8 Ft	5-6 Ft	6-7 Ft	8-9 Ft	14-15 Ft	7-8 Ft	8-9 Ft	5-6 Ft	6-7 Ft	6-7 Ft dup	4-5 Ft
PAH																
1-Methylnaphthalene	mg/kg	23	0.44	72	0.19	1.8	0.14	0.25	2.2	6.1	9	1.4	220	0.023	0.005	0.036
2-Methylnaphthalene	mg/kg	20	0.59	99	0.11	0.47	0.082	0.31	2.8	11	12	1.6	250	0.010	0.005	0.055
Acenaphthene	mg/kg	38	0.071	94	0.23	3.1	0.17	0.044	3.5	3.4	13	1.6	310	0.091	0.027	0.013
Acenaphthylene	mg/kg	0.7	0.019	5.2	<0.0097	0.49	0.072	0.03	0.2	15	9.9	0.79	25	0.012	<0.0075	0.028
Anthracene	mg/kg	3,000	0.096	40	0.024	2	0.082	0.13	1.8	9.5	10	1.3	150	0.023	0.009	0.047
Benzo(a)anthracene	mg/kg	17	0.084	20	0.022	1.7	0.10	0.24	0.79	9.6	13	1.3	89	0.055	0.019	0.11
Benzo(a)pyrene	mg/kg	48	0.058	15	0.027	1.4	0.15	0.17	0.58	13	30	1.1	67	0.058	0.019	0.12
Benzo(b)fluoranthene	mg/kg	360	0.048	6.5	<0.014	0.76	0.089	0.18	0.23	8.9	15	0.66	32	0.028	<0.011	0.088
Benzo(ghi)perylene	mg/kg	6,800	0.025	7.5	0.015	0.85	0.10	0.07	0.24	14	13	0.72	19	0.031	0.010	0.069
Benzo(k)fluoranthene	mg/kg	870	0.044	8.9	<0.019	0.86	0.097	0.17	0.36	8.9	15	0.78	41	0.035	<0.015	0.094
Chrysene	mg/kg	37	0.096	19	0.024	1.7	0.11	0.25	0.76	13	17	1.4	82	0.046	0.018	0.120
Dibenz(a,h)anthracene	mg/kg	38	<0.0084	1.7	<0.0058	0.2	0.017	0.24	0.061	2	3.3	0.15	5	0.007	<0.0044	0.015
Fluoranthene	mg/kg	500	0.015	46	0.43	3.5	0.19	0.5	1.9	17	18	2.7	190	0.099	0.039	0.210
Fluorene	mg/kg	100	0.056	33	0.054	1.1	0.073	0.054	1.2	2.3	4	0.83	130	0.024	0.004	0.012
Indeno(1,2,3-cd)pyrene	mg/kg	680	0.019	5.2	0.012	0.6	0.077	0.072	0.18	9.1	10	0.5	15	0.023	0.008	0.059
Naphthalene	mg/kg	0.4	0.50	200	0.430	0.71	0.12	0.23	4.5	190	22	5.1	160	0.038	0.013	0.056
Phenanthrene	mg/kg	1.8	0.52	130	0.10	6.70	0.33	0.51	5.2	15	22	4.3	520	0.015	0.008	0.160
Pyrene	mg/kg	8,700	0.19	63	0.053	4.3	0.18	0.37	2.4	19	28	3.8	260	0.140	0.051	0.170
VOC																
Benzene	mg/kg	0.0055	0.130	2.2	3.3	25	0.690	5.7	14	13,000	5.9	34	17	0.044	<0.033	1.2
Ethylbenzene	mg/kg	2.9	0.048	42	<0.025	5.6	0.063	0.3	16	170	0.73	31	3.2	<0.043	<0.033	<0.028
Toluene	mg/kg	1.5	0.27	1.3	<0.025	9	0.100	6.5	<0.2	11,000	3.2	1.0	0.16	0.53	0.12	0.42
Xylene, -o	mg/kg	4.1 ^b	0.11	16	<0.025	4.7	0.064	0.78	5.9	2,100	0.62	24	1.3	<0.043	< 0.033	0.038
Xylenes, -m, -p	mg/kg	4.1 ^b	0.19	29	<0.050	6.5	0.090	2.2	1.5	2,900	1.1	85	0.62	0.068	<0.033	0.07

a. Residual Contaminant Levels (RCL) for Protection of Groundwater. No RCLs established for metals.

RCLs for PAH are from Table 1, Soil Cleanup Levels for PAHs Interim Guidance. RCL for BTEX are from Table 1, WAC NR 720.

b. The RCL for xylene is 4,100 ug/kg for the sum of all xylenes.

Note: Bold results indicate concentrations greater than applicable RCL.

Approximate sample depth beneath sample ID is reported in feet below grade.

Only deep soil samples (greater than 4-feet deep) are reported in this table.

Samples were collected for the Phase II, Part III Investigation (see Phase II, Part III Report dated March 2005).

Table 2d Summary of Deep Phase II, Part IV Soil Analytical Results SWLP MGP Superior, Wisconsin

Sample ID	Unite	WDNR Soil	MW-14	MW-15	MW-16	MW-16 Dup	MW-21	MW-22	LIF-55	LIF-48
Sample Depth	Units	Standards ^a	0-7	4-5	6-7	6-7	4-5	4-5	0-6	4-5
Date			10/10/2005	10/11/2005	10/12/2005	10/12/2005	10/12/2005	10/13/2005	5/25/2006	5/25/2006
VOC										
Benzene	ug/kg	5.5	<53	120	<61	<50	<48	2,200		
Ethylbenzene	ug/kg	2,900	<53	<46	<61	<50	<48	<62		
Naphthalene	ug/kg	400	<53	110	<61	<50	<48	<62		
p-Isopropyltoluene	ug/kg	NE	<53	<46	<61	<50	<48	190		
Toluene	ug/kg	1,500	<53	75	71	<50	<48	820		
Xylene, o	ug/kg	4,100 ^b	<53	<46	<61	<50	<48	<62		
Xylenes, m + p	ug/kg	4,100 ^b	<110	<93	<120	<100	<96	<120		
PAH										
1-Methylnaphthalene	ug/kg	23,000	<3.9	48	16	15	14	14	24	17
2-Methylnaphthalene	ug/kg	20,000	<4.0	81	22	23	19	22	19	16
Acenaphthene	ug/kg	38,000	<3.8	31	4.9	<3.3	8.5	<4.5	51	<6.5
Acenaphthylene	ug/kg	700	<3.7	350	3.8	3.7	<3.1	<4.3	100	<6.3
Anthracene	ug/kg	3,000,000	<4.6	140	14	11	25	<5.4	130	<7.8
Benzo(a)anthracene	ug/kg	17,000	<6.9	270	37	28	53	15	340	<12
Benzo(a)pyrene	ug/kg	48,000	<3.7	590	37	26	45	8.5	440	11
Benzo(b)fluoranthene	ug/kg	360,000	<3.6	340	28	21	35	6	240	8.3
Benzo(ghi)perylene	ug/kg	6,800,000	<4.6	310	18	20	20	<5.4	240	7.8
Benzo(k)fluoranthene	ug/kg	870,000	<4.0	340	34	23	42	<4.6	350	7.3
Chrysene	ug/kg	37,000	<5.6	280	36	29	50	21	450	<9.6
Dibenz(a,h)anthracene	ug/kg	38,000	<3.6	77	6.1	6.3	7.2	<4.2	55	<6.0
Fluoranthene	ug/kg	500,000	4.8	31	60	44	100	8.8	400	<6.3
Fluorene	ug/kg	100,000	<4.4	21	6.3	4.1	8.3	<5.2	46	<7.5
Indeno(1,2,3-cd)pyrene	ug/kg	680,000	<3.3	240	18	17	21	<3.8	140	6
Naphthalene	ug/kg	400	<5.2	110	19	17	15	16	<21	<8.8
Phenanthrene	ug/kg	1,800	<3.8	230	61	43	88	15	200	<6.5
Pyrene	ug/kg	8,700,000	4.2	330	60	45	86	36	700	7.5

a. Residual Contaminant Levels (RCL) for Protection of Groundwater. No RCLs established for metals.

RCLs for PAH are from Table 1, Soil Cleanup Levels for PAHs Interim Guidance. RCL for BTEX are from Table 1, WAC NR 720.

b. The RCL for xylene is 4,100 ug/kg for the sum of all xylenes.

Note: Bold results indicate concentrations greater than applicable RCL.

Approximate sample depth beneath sample ID is reported in feet below grade.

Only deep soil samples (greater than 4-feet deep) are reported in this table.

Samples were collected for the Phase II, Part IV Investigation (see Phase II, Part IV Report dated May 2007).

Table 3Summary of Groundwater Analytical ResultsSuperior Water, Light Power MGPSuperior, Wisconsin

Well IL	Enforcement	MW-1	MW-2	MW-3	MW-4	MW-5	MW-6	MW-7	MW-8	MW-9	MW-10	MW-11	MW-11 FD	MW-12	MW-13	MW-13	MW-14	MW-14	MW-15	MW-15 FD
Date	, Standard ^a	11/15/2005	11/15/2005	11/15/2005	11/15/2005	11/15/2005	11/16/2005	11/16/2005	11/15/2005	11/15/2005	11/15/2005	11/15/2005	11/15/2005	11/14/2005	11/15/2005	10/24/2006	11/16/2005	10/24/2006	11/14/2005	11/14/2005
VOC																				
Acetone	1,000															<5.0		<5.0		
Benzene	5	<0.41	<0.41	2,800	190,000	<0.41	4.6	110,000	73,000	29,000	13,000	1.4	1.4	4,100	3.8	<1.0	<0.41	<1.0	23	21
2-Butanone (MEK)	460															<5.0		<5.0		
Bromobenzene	NE ^b	<0.82	<0.82	<20	<1,000	<0.82	<0.82	<820	<510	<200	<100	<0.82	<0.82	<20	<0.82	<1.0	<0.82	<1.0	<0.82	<0.82
Chloroethane	400	<0.97	<0.97	<24	<1,200	<0.97	0.97	<970	<610	<240	<120	<0.97	<0.97	<24	<0.97	<1.0	<0.97	<1.0	<0.97	<0.97
Chloroform	6	<0.37	<0.37	<9.2	<460	<0.37	<0.37	<370	<230	<92	<46	<0.37	<0.37	<9.2	<0.37	<1.0	<0.37	<1.0	<0.37	<0.37
Chloromethane	3	0.33	<0.24	<6.0	<300	<0.24	<0.48	<240	<150	<60	<30	0.25	<0.24	<6.0	0.6	<1.0	0.56	<1.0	<0.24	<0.24
Ethylbenzene	700	<0.54	<0.54	130	<680	<0.54	3.3	3,600	510	530	240	0.91	1	<14	<0.54	<1.0	<0.54	<1.0	6.8	5
Isopropylbenzene (Cumene)	NE	<0.59	<0.59	<15	<740	<0.59	<0.59	<590	<370	<150	<74	<0.59	<0.59	<15	<0.59	<1.0	<0.59	<1.0	4.3	4
p-Isopropyltoluene	NE	<0.67	<0.67	<17	<840	<0.67	<0.67	<670	<420	<170	<84	<0.67	<0.67	<17	<0.67	<1.0	<0.67	<1.0	<0.67	<0.67
Naphthalene	100	<0.74	<0.74	2,100	<920	1.2	26	<740	680	340	240	29	33	<18	<0.74	<1.0	0.93	<1.0	110	90
n-Propylbenzene	NE	<0.81	<0.81	<20	<1,000	<0.81	<0.81	<810	<510	<200	<100	<0.81	<0.81	<20	<0.81	<1.0	<0.81	<1.0	1.6	1.4
Styrene	100	<0.86	<0.86	<22	<1,100	<0.86	<0.86	<860	2,000	<220	<110	<0.86	<0.86	<22	<0.86	<1.0	<0.86	<1.0	<0.86	<0.86
Toluene	1,000	<0.67	<0.67	25	1,500	<0.67	1.1	57,000	51,000	6,700	5,100	<0.67	<0.67	<17	<0.67	<1.0	<0.67	<1.0	<0.67	<0.67
1,2,4-Trimethylbenzene	480 [°]	<0.97	<0.97	120	<1,200	<0.97	<0.97	<970	<610	<240	<120	<2.9	<3.0	<24	<0.97	<1.0	<0.97	<1.0	25	23
1,3,5-Trimethylbenzene	480	<0.83	<0.83	41	<1,000	<0.83	<0.83	<830	<520	<210	<100	<0.83	<0.83	<21	<0.83	<1.0	<0.83	<1.0	3.6	2.9
m&p-Xylene	10,000 ^d	<1.8	<1.8	260	<2,200	<1.8	<1.8	12,000	9,900	2,200	770	<1.8	<1.8	<45	<1.8	<2.0	<1.8	<2.0	<1.8	<1.8
o-Xylene	10,000 ^d	<0.83	<0.83	25	<1,000	<0.83	1.2	2,500	2,200	420	180	1.4	1.5	<21	<0.83	<1.0	<0.83	<1.0	2.8	2.2
PAH																				
Acenaphthene	NE	0.049	<0.0088	2.7	<0.086	0.38	5.1	3.1	37	39	38	8.7	9.6	46	<0.0086	<0.04	<0.0086	<0.04	43	51
Acenaphthylene	NE	<0.0086	<0.0088	1.4	<0.086	0.011	<0.43	1.3	4.7	1.6	2.9	0.1	0.11	<0.86	<0.0086	<0.04	<0.0086	<0.04	<1.7	0.71
Anthracene	3,000	<0.012	<0.013	1.7	<0.12	0.034	<0.61	<1.3	7.9	8.4	8.6	0.12	0.13	4.0	<0.012	<0.04	<0.012	<0.04	3.5	4.2
Benzo(a)anthracene	NE	<0.017	<0.017	<1.7	<0.17	<0.017	<0.83	<1.7	<1.7	<1.7	3.9	0.017	0.018	<1.7	<0.017	<0.04	<0.017	<0.04	0.27	<0.33
Benzo(a)pyrene	0.2	<0.019	<0.020	<1.9	<0.19	<0.019	<0.97	<2.0	<1.9	<1.9	2.7	0.019	<0.019	<1.9	<0.019	<0.04	<0.019	<0.04	0.11	<0.39
Benzo(b)fluoranthene	0.2	<0.017	<0.017	<1.7	<0.17	<0.017	<0.83	<1.7	<1.7	<1.7	<1.7	<0.017	<0.017	<1.7	<0.017	<0.04	<0.017	<0.04	0.054	<0.33
Benzo(g,h,i)perylene	NE	<0.020	<0.021	<2.0	<0.20	<0.020	<1.0	<2.1	<2.0	<2.0	<2.0	<0.020	<0.020	<2.0	<0.020	<0.04	<0.020	<0.04	0.054	<0.41
Benzo(k)fluoranthene	NE	<0.020	<0.021	<2.0	<0.20	<0.020	<1.0	<2.1	<2.0	<2.0	<2.0	<0.020	<0.020	<2.0	<0.020	<0.04	<0.020	<0.04	0.063	<0.41
2-Chloronaphthalene	NE															<0.04		<0.04		
Chrysene	0.2	<0.020	<0.021	<2.0	<0.20	<0.020	<1.0	<2.1	<2.0	<2.0	4.5	<0.020	<0.020	<2.0	<0.020	<0.04	<0.020	<0.04	0.22	<0.40
Dibenzofuran	NE															< 0.04		< 0.04		
Fluoranthene	400	< 0.016	<0.017	<1.6	<0.16	0.041	<0.82	<1.7	6.6	4.8	11	< 0.059	0.059	<1.6	<0.016	< 0.04	<0.016	0.057	<3.3	2.2
Fluorene	400	0.0097	<0.0098	6	<0.096	0.2	0.5	1.7	11	12	11	0.73	0.79	8.7	0.014	< 0.04	<0.0096	< 0.04	7.3	10
Indeno(1,2,3-cd)pyrene	NE	<0.020	< 0.020	<2.0	<0.20	<0.020	<1.0	<2.1	<2.0	<2.0	<2.0	<0.020	<0.020	2.0	<0.020	< 0.04	<0.020	< 0.04	0.037	< 0.40
1-Methylnaphthalene	NE	0.07	<0.012	82	0.11	0.14	4.1	6.2	61	42	41	9.4	9.9	43	0.055	< 0.04	<0.011	< 0.04	45	57
2-Methylnaphthalene	NE	0.05	<0.012	29	0.13	0.057	2.4	8.4	44	44	18	1.2	1.2	1.8	0.045	< 0.04	<0.012	< 0.04	17	20
Naphthalene	100	0.28	0.038	650	2.9	0.77	18	330	380	160	110	17	18	<4.9	0.34	< 0.04	0.023	< 0.04	83	93
Phenanthrene	NE	< 0.012	<0.012	9.6	<0.12	0.067	3.1	3.2	35	33	30	0.39	0.46	<15	0.022	< 0.04	<0.012	0.073	16	22
Pyrene	250	<0.015	<0.016	<1.5	<0.15	0.033	0.81	<1.6	8.6	6.3	15	0.085	0.089	<1.8	<0.015	<0.04	<0.015	0.068	<3.1	2.6

Notes:

Results are reported in micrograms per liter or parts per billion.

Shaded results indicate concentrations greater than the enforcement standards.

a. The Wisconsin Department of Natural Resources Groundwater Enforcement Standards

for the protection of public health (NR 140, Table 1). b. NE means enforcement standard is not established.

c. The enforcement standard is 480 ug/L for the sum of all trimethylbenzene concentrations.

d. The enforcement standard is 10,000 ug/L for the total xylene concentrations.

Table 3Summary of Groundwater Analytical ResultsSuperior Water, Light Power MGPSuperior, Wisconsin

Well ID	Enforcement	MW-15	MW-16	MW-16	MW-17	MW-17	MW-18	MW-18	MW-19	MW-19	MW-20	MW-20	MW-21	MW-21	MW-22	MW-22
Date	Standard ^a	10/24/2006	11/15/2005	10/24/2006	11/15/2005	10/24/2006	11/15/2005	10/24/2006	11/14/2005	10/24/2006	11/14/2005	10/24/2006	11/15/2005	10/24/2006	11/15/2005	10/24/2006
VOC																
Acetone	1,000	<5.0		<5.0		<5.0		110		<5.0		<5.0		<5.0		171
Benzene	5	23.2	<0.41	<1.0	<0.41	<1.0	4.1	4.1	<0.41	<1.0	3,800	<1.0	<0.41	<1.0	10	6.4
2-Butanone (MEK)	460	<5.0		<5.0		<5.0		<5.0		<5.0		<5.0		<5.0		10.5
Bromobenzene	NE ^b	<1.0	<0.82	<1.0	<0.82	<1.0	<0.82	<1.0	<0.82	<1.0	<41	5,380	<0.82	<1.0	<0.82	<1.0
Chloroethane	400	<1.0	<0.97	<1.0	<0.97	<1.0	<0.97	<1.0	<0.97	<1.0	<48	<1.0	<0.97	<1.0	<0.97	<1.0
Chloroform	6	<1.0	<0.37	<1.0	<0.37	<1.0	<0.37	<1.0	<0.37	<1.0	<18	<1.0	0.39	<1.0	<0.37	1.1
Chloromethane	3	<1.0	0.53	<1.0	<0.24	<1.0	0.33	<1.0	<0.24	<1.0	<12	<1.0	<0.24	<1.0	0.48	<1.0
Ethylbenzene	700	5	<0.54	<1.0	<0.54	<1.0	<0.54	<1.0	<0.54	<1.0	43	10.1	<0.54	<1.0	<0.54	<1.0
Isopropylbenzene (Cumene)	NE	4.4	<0.59	<1.0	<0.59	<1.0	<0.59	<1.0	<0.59	<1.0	<30	6.7	<0.59	<1.0	<0.59	<1.0
p-Isopropyltoluene	NE	<1.0	<0.67	<1.0	<0.67	<1.0	<0.67	<1.0	<0.67	<1.0	<34	<1.0	<0.67	<1.0	<0.67	2.3
Naphthalene	100	79.7	<0.74	<1.0	<0.74	<1.0	0.89	<1.0	<0.74	<1.0	280	41.1	<0.74	<1.0	2.7	2.9
n-Propylbenzene	NE	1.5	<0.81	<1.0	<0.81	<1.0	<0.81	<1.0	<0.81	<1.0	<40	3.1	<0.81	<1.0	<0.81	<1.0
Styrene	100	<1.0	<0.86	<1.0	<0.86	<1.0	<0.86	<1.0	<0.86	<1.0	<43	<1.0	<0.86	<1.0	<0.86	<1.0
Toluene	1,000	<1.0	<0.67	<1.0	<0.67	<1.0	3.2	1.1	<0.67	<1.0	<34	<1.0	<0.67	<1.0	1.5	1.8
1,2,4-Trimethylbenzene	480 ^c	17.7	<0.97	<1.0	<0.97	<1.0	<0.97	<1.0	<0.97	<1.0	<48	31	<0.97	<1.0	<0.97	<1.0
1,3,5-Trimethylbenzene	480	1.7	<0.83	<1.0	<0.83	<1.0	<0.83	<1.0	<0.83	<1.0	<42	1.3	<0.83	<1.0	<0.83	<1.0
m&p-Xylene	10,000 ^d	<2.0	<1.8	<2.0	<1.8	<2.0	<1.8	<2.0	<1.8	<2.0	<90	<1.0	<1.8	<2.0	<1.8	<2.0
o-Xylene	10,000 ^d	2.4	<0.83	<1.0	<0.83	<1.0	<0.83	<1.0	<0.83	<1.0	<42	12.6	<0.83	<1.0	<0.83	<1.0
PAH																
Acenaphthene	NE	49.6	0.042	<0.04	0.017	0.056	0.09	0.1	0.045	<0.04	14	27.1	0.016	<0.04	1.9	0.14
Acenaphthylene	NE	<0.04	<0.0086	<0.04	<0.0086	<0.04	0.013	<0.04	<0.0086	<0.04	<0.86	<0.04	<0.0086	<0.04	0.12	<0.04
Anthracene	3,000	2.8	0.023	<0.04	0.015	<0.04	0.049	0.072	0.015	<0.04	<1.2	0.2	<0.012	<0.04	0.98	0.05
Benzo(a)anthracene	NE	0.23	0.027	0.049	<0.017	<0.04	0.044	0.047	<0.017	<0.04	<1.7	<0.04	<0.017	<0.04	0.4	0.052
Benzo(a)pyrene	0.2	<0.04	0.021	<0.04	<0.019	<0.04	0.026	<0.04	<0.019	<0.04	<1.9	<0.04	<0.019	<0.04	0.21	<0.04
Benzo(b)fluoranthene	0.2	0.16	<0.017	0.17	<0.017	<0.04	0.019	0.15	<0.017	<0.04	<1.7	<0.04	<0.017	<0.04	<0.17	0.16
Benzo(g,h,i)perylene	NE	<0.04	<0.020	0.26	<0.020	<0.04	<0.020	<0.04	<0.020	<0.04	<2.0	<0.04	<0.020	<0.04	<0.20	0.26
Benzo(k)fluoranthene	NE	<0.04	<0.020	<0.04	<0.020	<0.04	<0.020	<0.04	<0.020	<0.04	<2.0	<0.04	<0.020	<0.04	<0.20	<0.04
2-Chloronaphthalene	NE	0.075		<0.04		<0.04		<0.04		<0.04		<0.04		<0.04		<0.04
Chrysene	0.2	0.19	0.024	0.044	<0.020	<0.04	0.044	<0.04	<0.020	<0.04	<2.0	< 0.04	<0.020	<0.04	0.38	0.057
Dibenzofuran	NE	0.61		<0.04		<0.04		0.042		<0.04		0.19		<0.04		< 0.04
Fluoranthene	400	1.9	0.035	0.097	0.023	<0.04	0.09	0.18	0.021	<0.04	<1.6	0.34	<0.016	<0.04	1.1	0.083
Fluorene	400	10.2	0.015	< 0.04	< 0.0096	<0.04	0.059	0.064	0.012	< 0.04	<0.96	3.3	<0.0096	<0.04	0.71	< 0.04
Indeno(1,2,3-cd)pyrene	NE	< 0.04	<0.020	< 0.04	<0.020	< 0.04	<0.020	< 0.04	<0.020	< 0.04	<2.0	< 0.04	<0.020	< 0.04	<0.20	< 0.04
1-Methylnaphthalene	NE	38.4	0.074	< 0.04	<0.011	<0.04	0.17	0.22	0.04	< 0.04	18	29.5	0.02	< 0.04	1.7	0.25
2-Methylnaphthalene	NE	9.4	0.047	<0.04	<0.012	<0.04	0.13	0.18	0.025	< 0.04	1.4	1.5	0.023	<0.04	1.2	0.17
Naphthalene	100	49.8	0.36	< 0.04	0.029	< 0.04	0.13	0.21	0.097	< 0.04	130	21.4	0.23	< 0.04	3.4	0.52
Phenanthrene	NE	14.9	0.054	0.075	0.052	0.07	0.21	0.43	0.036	< 0.04	<1.2	1.2	<0.012	< 0.04	3.1	0.21
Pyrene	250	2.5	0.059	0.079	0.037	<0.04	0.16	0.21	0.026	<0.04	<1.5	0.29	<0.015	<0.04	1.5	0.1

Notes:

Results are reported in micrograms per liter or p Shaded results indicate concentrations greater t

a. The Wisconsin Department of Natural Resour

for the protection of public health (NR 140, Ta

b. NE means enforcement standard is not estab

c. The enforcement standard is 480 $\mbox{ug/L}$ for the

d. The enforcement standard is 10,000 ug/L for

Well ID	Ground Elevation ^a	Measuring Point Elevation ^b	Depth to Water ^c	Groundwater Elevation ^b	Hydraulic Conductivity ^d
MW-1	616.2	619.11	9.67	609.44	Clay ^e
MW-2	614.2	617.15	6.11	611.04	Clay
MW-3	613.9	617.07	6.45	610.62	Clay
MW-4	614.0	617.11	7.31	609.80	Clay
MW-5	610.1	612.40	8.49	603.91	7.63 x 10 ⁻⁵
MW-6	611.4	613.74	10.02	603.72	3.07 x 10 ⁻³
MW-7	612.3	614.91	11.7	603.21	7.79 x 10 ⁻³
MW-8	612.0	615.17	12.06	603.11	3.26 x 10 ⁻³
MW-9	608.7	611.38	8.53	602.85	1.17 x 10 ⁻²
MW-10	606.5	606.08	3.55	602.53	7.46 x 10 ⁻³
MW-11	607.0	609.89	8.27	601.62	8.48 x 10 ⁻³
MW-12	607.9	607.64	5.86	601.78	3.28 x 10 ⁻³
MW-13	613.56	616.26	7.45	608.81	Clay
MW-14	614.06	617.27	8.70	608.57	Clay
MW-15	609.06	608.95	7.12	601.83	1.1 x 10 ⁻³
MW-16	610.03	613.11	10.20	602.91	1.6 x 10 ⁻³
MW-17	608.48	610.93	8.33	602.60	2.3 x 10 ⁻³
MW-18	606.4	606.42	2.67	603.75	4.5 x 10 ⁻⁵
MW-19	606.82	606.77	3.91	602.86	1.0 x 10 ⁻²
MW-20	605.91	605.43	4.22	601.21	6.8 x 10 ⁻³
MW-21	609.59	612.57	9.82	602.75	1.5 x 10 ⁻¹
MW-22	607.5	610.55	7.5	603.05	4.4 x 10 ⁻³

Table 4Groundwater Elevation Data and Well SummarySuperior Water Light Power MGPSuperior, Wisconsin

Groundwater elevations were measured on 10/24/06 with an interface probe.

a. The ground surface and top of casings elevations were surveyed by Salo Engineering.

- b. Elevation is given in feet above mean sea level.
- c. Depth to water in feet as measured below top of casing.
- d. Hydraulic conductivity (cm/sec) was determined by conducting slug tests in November 2001, November 2004, and October 2006.
- e. Wells screened in high plasticity clay. Estimated hydraulic conductivity is less than 10⁻⁶ cm/sec. (Slug test was not performed on well.)

Appendix A Well and Geologic Records

	ng-14W				· •	
	14,41	 E. A. BIRGE, I	Director and	 Superintende	nt.	
	NW E. R. BU	CKLEY, Geolo	gist, in chei	rge of Econom	ic Geology,	
320	NE					
	ST2	ATISTICAL D	ATA OF V	ELL BORIN	G S	
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						•••••••••••••••••••••••••••••••••••••••
Is the	ell situated on a hill? If so giv	e approximate ele	vation	ca fil	i m	lakeor
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I	38p. 31V			· · · · · · · · · · · · · · · · · · ·		
S	allow		• • • • • • • • • • • • • • • •			
Not f	wing:					
I	ер	****	· · · · · · · · · · · · · · · · · · ·			
s	allow		••••••	****		••••
Total de	oth of well from surface			••••••••••••••••	feet	
Depth o	well from surface to where rock	k is first reached.	· · · · · · · · · · · · · · · · · · ·	• • • • • • • • • • • • • • • • • • • •	feet	
Charact	r of material passed through al	love rock	20 f1-C	lug	0	······
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	ROT.V		1 10	Auce	el	2
····	an more	ace ita	ra peun	- lust	Soft-H	un mas
• • • • • • • •	and the set	isface	nan	waler	would	mee to
Total th	ckness of rock passed through.	••••••••••••••••••••	••••••		feet	
Kinds of	rock and thickness of each, beg	inning at the top:				· · · ·
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. 20	kind	•••••••••••••••••••••••••••••••••••••••	••••••••••		depth in feet	·
36	kind			••••••••••••••••••••••••••••••••••••••	depth in feet	• •••••••••••••••••••••••••••••••••••••
46	king			•••••••••••••••••	depth in feet	

This Will Flows 144-our
luse conface level
Amount of flow of well at 6 ft above 2 gallons per minute
Depth of water in well. 3.1.
Have you a chamical analysis of the water? had
C Dow 1 1.
What was the method employed in drilling the well?
· · · · · · · · · · · · · · · · · · · ·
What was the price of drilling?
Above the rock
In the rock per foot
Give location of important springs in or near your city
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······································
Check below the source of the water supply of the city or town in which you live
Waterworks: JEO
River or creek
Lake Lupenor
Spring
Well
No waterworks
Return to E. R. BUCKLEY, Madison, Wis.

Dated.

2234-2

建設 建合金

Property FIFL D L OOLO			Teleph	one		Madison, WI 537	07	De	epth 56	FT
Owner FIELD LOGIC			Numb	er		1. Well Locatio	n V=Village		Firo#	
ddress 101 MAIN ST						C of SUPER	IOR		ruc#	
		State V	/I Zip C	ode 5	4880	Street Address or 1 101 MAIN ST	Road Name and	l Number		
ounty of Well Location	Co Well	Permit No	Well	Completion D	ate	Subdivision Name		Lot#	Block #	
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ddress 0945 E HWY 2			Public We	ll Plan Approv	val#	Latitude	Deg. e Deg	Mın. Min.		
	State Z	ip Code	Date Of A	pproval		2. Well Type	1 (See item 12 belo	w) Lat/I	ong Meth
icap Permanent Well #	Common W	24854 /ell #	Specific C	anacity		1=New 2=R	eplacement 3	=Reconstruction	1 	
			1.8	gpm/ft		of previous unique	e well #	constructe	d in	-
Vell Serves # of homes and o	r INDUSTR	RY NON P		High Capa Well?	city: N	Reason for replace	d or reconstruct	ted Well?		
Munic O=OTM N=NonCom P=Private Z=Othe	er X=NonPot A=.	Anode L≖Loop	H=Drillhole	Property?	N	1 1=Drilled 2=D	riven Point 3=1	etted 4=Other		
the well located upslope or sideslop	e and not dov	wnslope fror	n any conta	mination sourc	ces, includin	ng those on neighbori	ng properties?	Y		
ell located in floodplain? N ance in feet from well to nearest: (inc	cluding propo	sed)	9. I)ownspout/ Ya	ard Hydrant		17. י	Wastewater Sum	p	
1. Landfill			10. 1	Privy Zoundation Dw	-i- to Class		18. 1	Paved Animal Ba	arn Pen	
2. Building Overhang			11. 1	Foundation Dra	ain to Clear	water	19. /	Animal Yard or S	Shelter	
3. 1=Septic 2= Hold	ling Tank		12. 1	Building Drain			20. 3	Silo Barn Guttar		
4. Sewage Absorption U	nit		400	1=Cast Ire	on or Plastic	c 2=Other	21. 1	Manure Pine	1=Gravity	=Precour
5. Nonconforming Pit	- Oil Tamle		100 14. 1	Building Sewer	r 1 1=Grav	vity 2=Pressure	22. 1	1=Cast iror	1 or Plastic	2=1 tessure 2=Other
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Owner Sent Label? Y More Geology?

WELL CO	NSTRUCTO	R'S REPORT		DEPARTA	STA	TE OF WI	SCONSIN	/ELOPM	ENT		₩/al_&
L COUNT	Y			CHECK	ONE		NAXE				
10m	- Acar			Town		e 🗇 Cit	v L	1	•		
2. LOCATIO	ON (Number	und Street or ½	section, se	stion, township	and range. A	lao give subd	ivision name, le	ot and block	numbers when a	wailable.)	
	<u> </u>	9 N	Reta	RI4W	Lea 1	4 7	7 A. Co	1 men	. of NE	? Pari	*
8. OWNER	AT TIME OF	DRILLING	61	2	1 0	, ,				/	
A OWNER	S COMPLET	MATT. ADDR	100	onald	Sol	erte	y			*	
				318	91.7	a.e.	W.	hl.	+4.12	the states of th	
5. Distanc	e In feet fr	om well to	nearest:	BUILDING SA	NITARY SEV	VER FLOOR	DRAIN	FOUNDATT	ION DRAL	WASTE W	ATER DRAIN
(Record a	nswer in appr	ropriate block)		11	C. L. TIL	E C.I.	TILE SEWER	CONNECTI	EDINDEPENDEN	T C.I.	TILE
OF BAD WH				30: 2	501	451			1 jun	1 45	1
CLEAR WA	TILE	SEPIIC TAN	K PRIVY	SEEPAGE PIT	ABSORPT	ION FIELD	BARN SI	LO ABAI	NDONED WELL	SINK HOLE	
45'		100'			/ /	251	·		/	-	
OTHER POI	LUTION SO	URCES (Give	description	such as damp,	quarry, drain	nage well, at	ream, pond, lak	e, etc.)	<u>y </u>		<u>.</u>
6 Wall In	intended	to supply a		aske o	2 50'						
	mended	io soppiy i		Hom							
7. DRILLHO	OLE					10. FOR	MATIONS				
Din. (in.)	From (ft.)	To (ft.)	Dia. (in.)	From (ft.)	To (ft.)	<u> </u>	Kind			From (ft.)	To (ft.)
10'	Surface	40	<u>4*</u>	150	305	Ber	l Clay			Surface	20
6'	40	150'				sands	x Grand	il (dr	y).	20	40
8. CASING	G, LINER, C	URBING, AN	ND SCREE	N	1	no		1	,,	Ha	10.
Dia. (in.)	011	and and Weight	9-45 P.E	From (ft.)	To (ft.)	cray	4 sil	ζ		70	150
6"	Std 6	20#2	the	Surface	150	Bed	Clay			150	290
4	St. 4"	11 to	J.	150	305	Haro	(Pan			290	300
						H				2	3.5
BH/424		·		-		14/00	uer .			300	403
							· · · · · · · · · · · · · · · · · · ·	······			
9. GROUT	OR OTHER	R SEALING	MATERIA		T. (6.)						
2	N	<u>nu</u>			10 (П.)						
· Pu	Alled	Clay		Surface	40						
		1				Well con	struction co	mpleted	on /	lept 3	1968
11. MISCE Yield test:	LLANEOUS	DATA	8 Hrs.	at ,	<i>10</i> gpm	Well is t	erminated	8	Inches	above below fi	inal grade
Depth from	n surface to	o normal wa	atar level		50 ft.	Well dis	infected upo	on comple	etion	Yes	, <u>∏</u> No
Depth to w	vater level	when pump	Ing		60ft.	Well sea	led watertig	iht upon	completion	Yes	I □ No
Water sam	ple sent to	m	adu	ion			J.	aboratory	ОП:	۲ ا	1968
Your opini wells, scre surface pu	ion concerr ens, seals, mprooms, a	ning other (type of ca access pits,	pollution ising joir etc., sho	hazards, in has, method uid be give	formation of finishi n on reve	concernin ing the w	g difficultie ell, amount	s encount of ceme	tered, and da nt used in gr	ta relating outing, bla	to nearby sting, sub-

SIGNATURE	····· · · · · · · · · · · · · · · · ·	COMPLETE M	ALL ADDRESS		
M. H. Jong	Registered Well I	Driller Poplo	u-liis.		
	Please do	not write in space i	below		· · · · · · · · · · · · · · · · · · ·
COLLFORM TEST RESULT See letter driller's file	GAS - 24 HRS. 4/14/68	GAS - 48 HRS.	CONFIRMED	REMARKS	
2235	1	l	1	I	

D5-4-1 sec. 14, T49N, R14 N, SW, NE. WELL CONSTRUCTOR'S REPORT TO WISCONSIN STATE BOARD OF HEALTH See Instructions on Reverse Side 15-4-4 Town putte Lper IOR County Village 🗍 City Ch 2. Location Name of stre and Bange nu 8. Owner 🗋 or Agent 🗍 🗕 SAUTTOTIC 4. Mail Address Complete address remined 5. From well to nearest: Building_ ft; sewer_____ft; drain___ff; septic tank_____ft;____ dry well or filter bed_____ft; abandoned well_____ft. water + Cooling 6. Well is intended to supply water for: ______ 7. DRILLHOLE: **10. FORMATIONS:** Dia. (In.) | From (ft.) | To (It.) Dia. (in.) 1 From (ft.) To (ft.) from ((t.) To (L) Kind 600 \bigcirc 25 25 70 0 8. CASING AND LINER PIPE OR CURBING: 105 Dia. (In.) . Y. Kind and Weight From (ft.) To (ft.) 105 130 140 153 30 969 5-5 600 m 269 9. GROUT: 269 60 0 'n Kind From (ft.) To (IL) ·) · Construction of the well was completed on: 11. MISCELLANEOUS DATA: Yield test: 25 Hrs. at 20 GPM. The well is terminated _____ _ inches \square show \square the permanent ground surface. Depth from surface to water-level: ____ $\mathcal{H}Q_{-}$ ft. Was the well disinfected upon completion? Water-level when pumping: _____ ZO_O_ft. Yes____No___ Water sample was sent to the state laboratory at: Was the well sealed watertight upon completion? _ 19____ Yes..... No..... Signature Registered Well Driller **Complete Mail Addres** Please do not write in space below 10 ml 10 ml 10 ml 10 ml 10 ml No. Rec'd Gas-24 hrs. Ansd 48 hrs. Interpretation Ч, Confirm 2236 B. Coll Examiner___