

ENVIRONMENTAL ASSESSMENT CITY OF ASHLAND, WISCONSIN WASTEWATER TREATMENT PLANT SITE

8/189

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ENVIRONMENTAL ASSESSMENT CITY OF ASHLAND ASHLAND, WISCONSIN WASTEWATER TREATMENT PLANT SITE

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Prepared For:

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Northern Er	nvironmental	Investigation	-	Results
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semi-volitals/ PAHs (ppb)	MW -1	MW-2	MW-3	composite soil
Acenaphthene	330	300	88	4400
Fluorene	180	150	83	4900
Phenanthrene	370	300	170	11000
Anthracene .	110	320	100	710
Fluoranthene	110	140	260	15000
Ругеле	230	370	790	35000
Benzo(a)anthracene	59	83	180	10000
Chrysene	60	93	180	10000
Bis (2-Ethylhexel) phathalate	18*	28	58	nd
Benzo(b)fluoranthrene	53	77	150	8000
Benzo(a)pyrene	51	79	170	8900
Indeno(1,2,3-cd)pyrene	22	33	80	3500
Dibenz(a,h)anthracene	nd	14*	26	1400
Benzo(g,h,i)perylene	27	45	110	4700
Phenol	nd	14*	nd	nd
Napthalene	150	1600	16*	3300
Ancenaphthylene	38	33	nd	1600
Volatiles (ppb)				
Benzene	190	390	51.4	nd
Ethylbenzene	nd	190	nd	nd
Toluene	27.1	31.7	nd	nd
m- and p-xylene	73.8	130	nd	nd
o-Xylene	150	220	nd	nd
Metals (ppb)				
Arsenic	nd	nd	nd	100000
Chromium	nd	nd	nd	5000
Copper	23	nd	nd	46500
Zinc	12	4	16	16000

ppb - Parts per billion

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semi-volitals/ PAHs (ppb)	MW-1	MW-2	MW-3	composite soil
Acenaphthene	330	300	88	4400
Fluorene	180	150	83	4900
Phenanthrene	370	300	170	11000
Anthracene	110	320	100	710
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Indeno(1,2,3-cd)pyrene	22	33	80	3500
Dibenz(a,h)anthracene	nd	14*	26	1400
Benzo(g,h,i)perylene	27	45	110	4700
Phenol	nd	14*	nd	nd
Napthalene	150	1800	16*	3300
Ancenaphthylene	38	33	nd	1600
Volatiles (ppb)				
Benzene	190	390	51.4	nd
Ethylbenzene	nd	190	nd	nd
Toluene	27.1	31.7	nd	nd
m- and p-xylene	73.8	130	nd	nd
o-Xylene	150	220	nd	nd
Metals (ppb)				
Arsenic	nd	nd	nd	100000
Chromium	nd	nd	nd	5000
Copper	23	nd	nd	46500
Zinc d - not detected * - estimated	12	4	16 Parts per b	16000

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Northern Environmental Investigation - Results

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nd - not detected * - estimated concentration ppb - Parts per billion

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1.0 EXECUTIVE SUMMARY

As part of a proposed expansion project for its wastewater treatment plant (WWTP), the City of Ashland conducted a feasibility study of the existing WWTP site. A soil exploration drilling program and environmental assessment of the property were conducted to evaluate soil geotechnical aspects and identify potential soil and/or ground-water impacts that may exist at the site.

The existing WWTP property occupies approximately 10 acres, with about one quarter of the property covered by structures. The property is currently owned and operated by the City of Ashland as the wastewater treatment facility for the city. A portion of the property was reportedly used as a disposal area for wood, wood preservatives, and electrical power transformers possibly containing polychlorinated biphenyls (PCBs).

The firm of Bonestroo, Rosene, Anderlik & Associates (Bonestroo) was retained to provide engineering and design services for the WWTP expansion. Bonestroo located and arranged for drilling of five geotechnical boreholes to be completed to 60 feet. The Citv Ashland retained Northern Environmental Technologies, of Incorporated (Northern Environmental) to provide an environmental assessment of the proposed expansion property. Northern Environmental completed an existing data review, a magnetometer observed geotechnical drilling and conducted an survev, environmental soil exploration drilling program to assess potential site soil and ground-water impacts. Three permanent stainless steel ground-water quality monitoring wells were also. installed in the environmental boreholes as part of this effort.

Six lithostratigraphic units were identified from information gathered during the investigative program. These units are presented in descending order below.

- 1) Upper Soil Fill
- 2) Wood Waste Layer
- 3) Beach Deposit
- 4) Lacustrine Deposit
- 5) Glacial Till
- 6) Glacial Outwash

Two distinct ground-water flow systems were identified in the upper 61 feet of sediments investigated at the Ashland site: a shallow water table system residing in the wood waste layer and the underlying discontinuous beach sand deposit; and a confined (artesian) aquifer residing in the glacial outwash. Hydraulic communication between the two flow systems is limited by a thick sequence of lacustrine clay and glacial till. A strong upward vertical hydraulic gradient (estimated to be 1.5 feet/feet) was measured at the site at an on-site flowing artesian well and flowing artesian geotechnical borings.

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Horizontal hydraulic gradients in the shallow ground-water flow system are extremely low (0.0001 foot/foot towards Lake Superior) and appeared to coincide with the water level in Lake Superior. More water level data would be necessary to confirm this information.

Data from geotechnical and environmental borings indicate that a creosote impacted wood waste layer of variable thickness exists at the existing WWTP site. The occurrence of the water table within the wood waste layer has caused dissolved and suspended constituents of creosote to enter the ground water. Soil and ground-water samples from the wood waste zone were submitted for laboratory analysis of PCBs, base neutral and acid extractable (BNA) compounds, volatile organic compounds (VOCs), selected metals, and waste water indicator parameters. These analysis detected elevated concentrations of BNA compounds that are characteristic constituents of creosote and VOCs which are characteristic compounds of lighter grade petroleum fuels. Concentrations of several parameters exceed State of Wisconsin soil/ground-water quideline, preventative action limits, and enforcement standard concentrations.

The lateral extent of impacted soil and ground water could not be fully defined in this study. However, impacts appear to be limited to filled areas south of the existing WWTP. The eastwest extent of these impacts could not be quantified. The vertical extent of soil/ground-water impacts appeared to be limited to the wood waste layer and underlying layers of lacustrine sand.

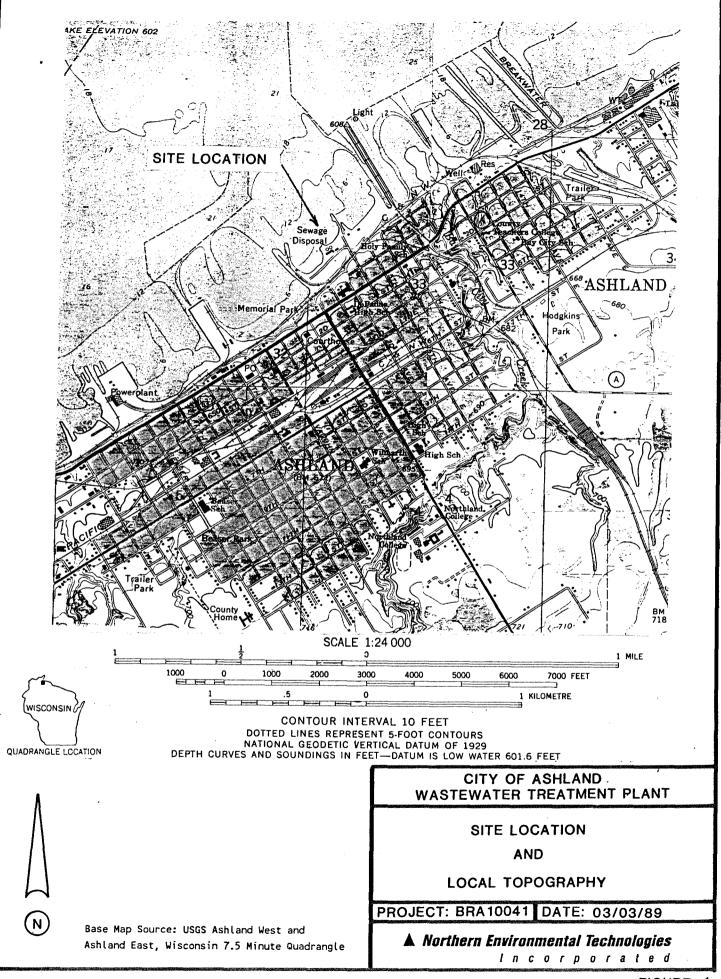
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2.0 INTRODUCTION

As part of a proposed expansion of its wastewater treatment plant (WWTP), the City of Ashland conducted a feasibility study at the existing WWTP site. The potential expansion area lies adjacent to the western and southern borders of the existing WWTP facility. The existing WWTP property occupies approximately 10 acres in the City of Ashland on Chequamegon Bay (NW; of Section 33, Township 48 North, Range 4 West; Latitude 46° 35′ 43", Longitude 90° 52' 59") (Figure 1). Approximately one quarter of the existing WWTP property is currently covered by structures. Α portion of the property was reportedly used as a disposal area for wood, wood preservatives, and possibly electrical power transformers potentially containing polychlorinated biphenyls (PCBs).

The firm of Bonestroo, Rosene, Anderlik & Associates (Bonestroo) was retained by the City of Ashland to provide engineering and design services for the proposed WWTP expansion. The proposed expansion would have been partially constructed on the suspected wood waste and transformer disposal area. Bonestroo located and arranged for drilling five 60 foot deep geotechnical boreholes. Drilling was performed by Twin City Testing, Incorporated. The City of Ashland retained Northern Environmental Technologies (Northern Environmental) to provide an environmental assessment of the proposed expansion property in conjunction with the geotechnical exploration program. Three additional shallow borings were located by Northern Environmental for environmental purposes.

The environmental assessment was designed to provide information concerning potential environmental liability associated with past present land use practices which could impact site soils and/or ground water and hence impede site development. A Northern Environmental hydrogeologist observed or directed the subsurface investigations which consisted of five deep foundation borings and three shallow environmental borings. The environmental borings were subsequently developed into three ground-water monitoring wells. This study presents the results of this effort and assesses the significance of the findings.



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3.0 METHODS OF INVESTIGATION

3.1 Existing Data Review

Existing sources of data were reviewed to identify land use practices and activities which may have potentially impacted site soil and/or ground-water quality. Additionally, this information was used to guide the placement of soil borings and subsequent field investigations. Northern Environmental interviewed City of Ashland employees at the WWTP and City Hall to assess past land use practices at and near the site (Reference 1).

3.2 Magnetometer Survey

A reconnaissance level magnetometer survey was conducted over the suspected disposal area to assess the presence of buried ferromagnetic objects in January, 1989. A Schoenstedt Magnetic Locator Model 52B was used to perform the survey. Access problems created by snow depths exceeding two feet limited the survey to five 100 foot long parallel east-west traverses over the suspected disposal area. Approximate spacing between traverses was ten feet.

3.3 Near Surface Soil Screening and Sampling

This task was omitted from the field investigations because of heavy snow cover and deeply frozen soils existing during the field investigation.

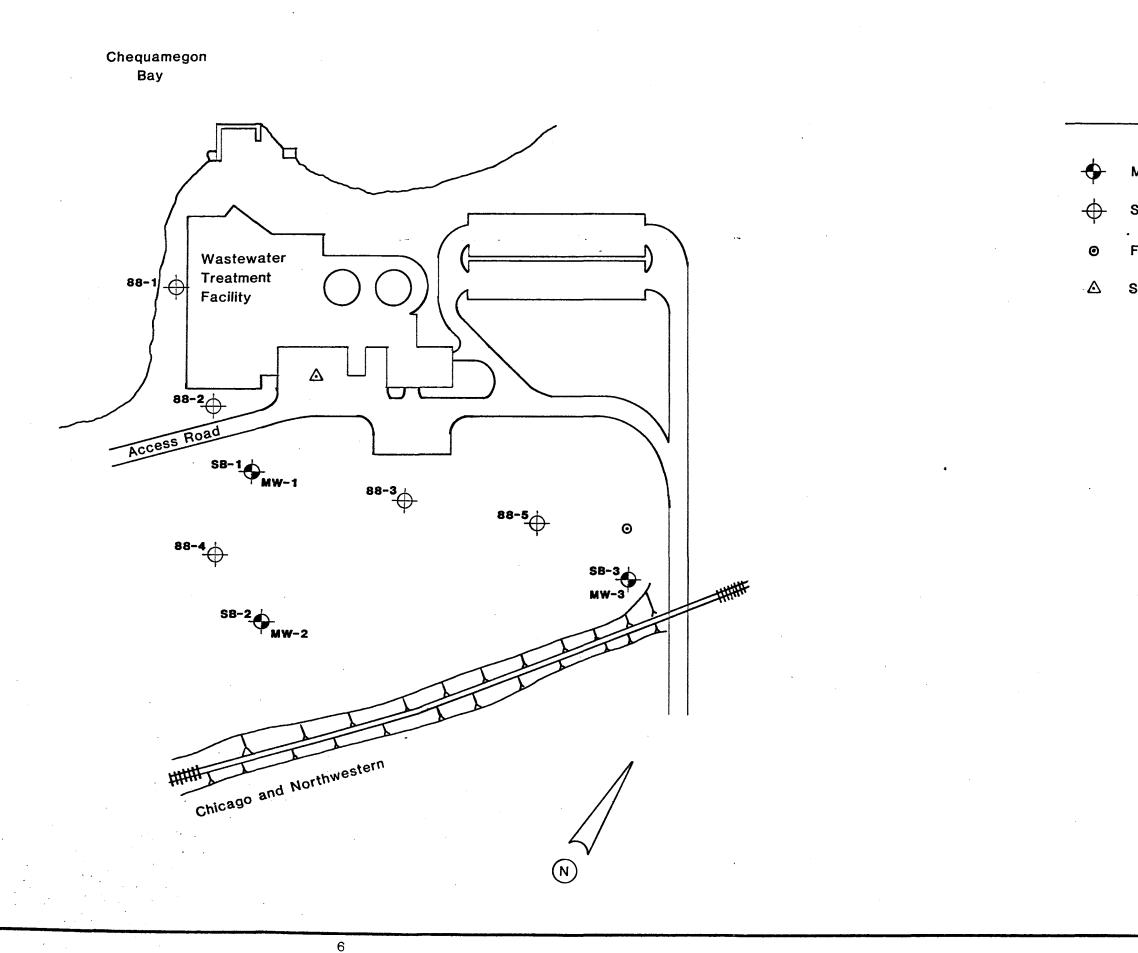
3.4 Soil Exploration, Drilling, and Sampling

The original objectives of the investigative program were to complete five deep geotechnical soil borings for foundation analysis and three environmental borings to assess soil impacts. The geotechnical and environmental borings were to be drilled to a total depth of 60 feet and 15 feet, respectively. The geotechnical boring locations were specified by Bonestroo and the environmental borings were located by Northern Environmental within the suspected disposal area (Figure 2).

3.4.1 Geotechnical Borings

Five geotechnical borings (88-1, 88-2, 88-3, 88-4, and 88-5) were drilled at the locations shown in Figure 2 in January, 1989. Three borings (88-1, 88-2, and 88-3) were drilled to 61.0 feet and two borings (88-4 and 88-5) were drilled to 41.0 feet. Borings 88-4 and 88-5 were abandoned at 41.0 feet because of problems associated with caving saturated sand and gravels and flowing artesian conditions.

Drilling at the first geotechnical borehole (88-2) was temporarily halted when a short interval of saturated wood waste was encountered at 12.1 - 12.4 feet. At this juncture, measures



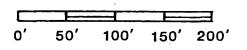
LEGEND

MONITORING WELL

SOIL BORING

FLOWING ARTESIAN WELL

SITE BENCHMARK



CITY OF ASHLAND WASTEWATER TREATMENT PLANT

SOIL BORING AND MONITORING

WELL LOCATIONS

PROJECT: BRA10041 DATE: 03/03/89

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were taken to avoid spreading near surface soil impacts to deeper permeable zones and/or aquifers. Northern Environmental recommended that any geotechnical borings that penetrated the saturated impacted zone be installed with a surface casing and grout seal to isolate this zone from deeper sediments. Borings 88-2, 88-3, 88-4, and 88-5 penetrated impacted zones and thus were installed with surface casing to depths varying from 14 to 18 feet. No casing was installed at 88-1 since no significant impacts were observed during drilling.

Geotechnical and environmental boreholes were advanced with a $3\frac{1}{2}$ -inch inside diameter (ID) hollow-stem auger (HSA) [$6\frac{1}{2}$ -inch outside diameter (OD)] in conformance with American Society of Testing and Materials (ASTM) standard method 1452. All downhole drilling equipment was steam cleaned between drilling locations. No lubricants were used on downhole drilling or sampling equipment. With the exception of boring 88-5, soil samples were collected every two feet to 15 feet and every five feet thereafter using standard split barrel techniques (ASTM 1586). Boring 88-5 was sampled continuously from 4.5 - 15.0 feet to ascertain the exact thickness of wood waste fill. Auger cuttings were examined during all phases of drilling at all locations. Downhole soil sampling equipment was washed with trisodium phosphate and double rinsed with potable water between subsequent samples.

Three different drilling bits were used at the WWTP site. The $3\frac{1}{4}$ -inch ID HSA and bit was the standard bit used, unless surface casing was installed. Cased boreholes were drilled from the base of surface casing to completion depth with a 2 7/8-inch tri-cone bit. When necessary, a $2\frac{1}{4}$ -inch carbide tipped drag bit was utilized to penetrate large cobbles or boulders.

A representative portion of each split barrel sample was collected and subjected to photoionization detector (PID) headspace screening to evaluate the possible presence of volatile and semivolatile organic compounds such as those which are found in creosote, petroleum fuels, and organic solvents. Screening was performed by immediately transferring approximately two inches of each split barrel sample to a small plastic storage bag, disaggregating the soil sample, and allowing the sample to degas in a warm (i.e. 50° F) location for at least 30 minutes. The bag was then punctured with the PID probe extension, and the highest stable PID reading measured within ten to twenty seconds was recorded. The headspace gas within each bag was analyzed for photoionizable constituent content utilizing a HNu Model GP101 PID outfitted with a 11.7 eV probe calibrated for direct response to benzene.

Soil samples that exhibited strong odors, high PID headspace readings, or which appeared visually impacted were stored in clean two ounce glass soil sample jars for potential laboratory analysis. Two soil samples (S6 from boring 88-2, and auger

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cuttings from boring SB-1) were field analyzed for gross polychlorinated biphenyl (PCB) content utilizing a Dexsil PCB screening kit. The Dexsil kit is a qualitative field analysis kit which detects the presence of PCBs and other chlorinated organic compounds to an approximate detection limit of ten parts per million (ppm).

One soil sample was composited from samples collected from borings 88-2 and 88-5 (DB#2/DB#5). This sample was submitted under chain of custody to a contract analytical laboratory approved by the Wisconsin Department of Natural Resources (WDNR). The soil sample underwent analysis for base, neutral, and acid extractable compounds (BNAs) (EPA Method 625). A second soil sample was selected from samples collected from boring 88-4 (DB#4) and was submitted for laboratory analysis of volatile organic compounds (VOCs) (EPA Method 601), polychlorinated biphenyls (PCBs), and selected metals. The results of these analyses are included in Appendix A1.

A portion of each split barrel sample and auger cuttings were collected and texturally described in the field by a Northern Environmental hydrogeologist. Final borehole logs were prepared by a Northern Environmental hydrogeologist in conformance with ASTM 2488. These logs include information on soil type, structural characteristics, consistency, density, lithology, photoionizable constituent content, estimated group symbol, and genetic origin. Copies of these logs can be found in Appendix B1. Borehole logs prepared by the drilling contractor are included in Appendix B2.

Upon completion of soil exploration drilling [(41.0 feet at borings 88-4 and 88-5, and 61.0 feet at borings 88-1, 88-2, and 88-3]) each boring was properly abandoned. All the geotechnical borings penetrated a saturated gravelly clayey sand confined outwash unit at 25 to 30 feet, and, because of strong upward vertical hydraulic gradients, became flowing artesian borings during or soon after drilling. The portion of the borehole below the outwash unit was tremied with cement/bentonite grout. Α bentonite plug was then utilized to obstruct the upward flow of ground water. The plug was positioned immediately above the confined outwash unit. Enough cement/bentonite grout was then pumped through the drill stem (tremied) above the plug until the grout had filled the borehole. This abandonment procedure was used on all geotechnical borings. The temporary 6-inch steel casings were not removed after abandonment. However, these casings can be removed at a later date for construction purposes if desired.

3.4.2 Environmental Borings

Three shallow (16.0 feet) environmental borings (SB-1, SB-2, and SB-3) were drilled in the suspected disposal area using hollow stem auger techniques as described in Section 3.4.1. As drilling

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progressed, the soils were sampled using standard split barrel techniques (ASTM 1586). Portions of each sample was screened with a PID as described in Section 3.4.1. The depths to top and bottom of wood waste fill were observed and noted on each boring log. Descriptive analysis of the wood waste included: strength and type of odor, color, size of fragments and degree and type of saturation. Soils were analyzed as described in Section 3.4.1. Final borehole logs were prepared by a Northern Environmental hydrogeologist in conformance with ASTM 2488 (Appendix B1). Borehole logs generated by the drilling contractor are included in Appendix B2.

3.5 Ground-Water Monitoring Well Installation

In view of the soil impacts discovered in boring 88-2, the scope of field investigations was broadened to evaluate any groundwater impacts associated with the the identified wood waste. The expanded scope included the installation and water quality sampling of three ground-water quality monitoring wells (MW-1, MW-2, MW-3), which were constructed in borings SB-1, SB-2, and SB-3, respectively.

To evaluate shallow ground-water quality and flow direction, three two-inch diameter stainless steel monitoring wells were installed immediately south of the WWTP (Figure 2). The wells were positioned to provide adequate lateral coverage for water level measurement and therefore provide adequate information to determine ground-water flow direction. Additionally, an attempt was made to locate each well beyond the proposed construction area. Since each well was completed within wood waste strata, none of the wells are considered representative indicators of upgradient ground-water quality.

To maintain borehole integrity during installation, wells were constructed within the 64-inch OD hollow stem auger. Auger flights were then removed one at a time. A measuring tape and weight were used to gauge the positions of natural sand pack, artificial sand pack, and bentonite seals.

All ground-water monitoring wells were constructed of two-inch diameter, threaded, flush joint, schedule five stainless steel casing with 0-ringed joints. Well screens consisted of one ten foot section of two-inch diameter, 0.010-inch continuous slot, threaded, flush joint stainless steel screen with 0-ringed joints. Bottom caps consisted of threaded flush joint stainless steel plugs. Top caps consisted of two-inch diameter PVC slip caps. Figure 3 illustrates typical monitoring well construction. Complete well construction summaries can be found in Appendix C.

The screened interval of each well was positioned to monitor the water table surface and the wood waste zone. In order to allow the presence of any floating product layer to be detected, the top of screen was positioned one-half to one foot above the measured water level on the day of installation. The depth to top of screen for wells MW-1, MW-2, and MW-3 are 4.0, 3.5, and

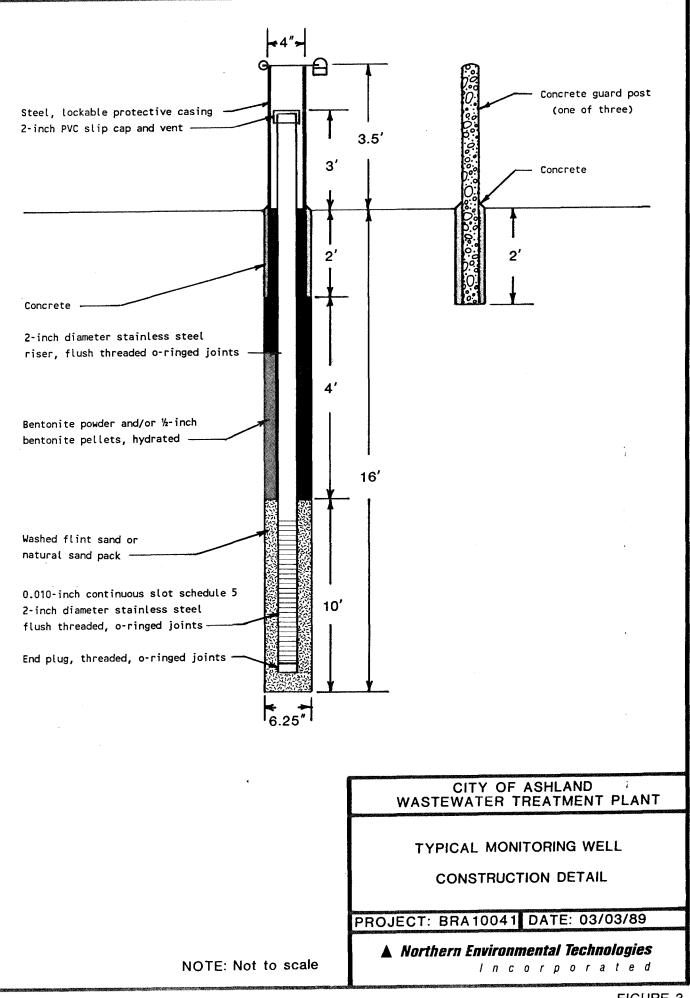


FIGURE 3

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5.0 feet, respectively. The annular space around all well screens was filled with medium to coarse grained clean flint sand. In well MW-3, the caving of silty fine sand sediments from 10.5 – 15.0 feet produced a natural sandpack for the lower five feet of screen. Flint sand was then added to the remaining screened annulus to one foot above the top of screen. A $1\frac{1}{2}$ to two foot thick bentonite seal was placed above the sandpack to provide an impermeable annular seal. This seal consisted of bentonite pellets for MW-3, powdered bentonite for MW-2, and a layer each of bentonite pellets and bentonite powder for MW-1.

To deter vandalism and prevent damage by vehicles, well protective casings and concrete filled guard posts were installed. Protective casing consisted of a five foot long, four by four inch square steel pipe. The protective casing was installed over the stainless steel riser and pushed two feet into the recently mixed concrete surface seal. A hinged cap fitted with a padlock provides access to the well head for water level measurements and sample recovery. Three guard posts per well were emplaced on a 120 degree radial spacing around the well to protect against vehicle impact. A small concrete pad was constructed and sloped to drain surface water away from the well.

Locations and elevations of all wells were determined after installation. The elevation of all water level reference points, riser pipes, protective casings, and natural ground surface were measured to the nearest 0.01 foot and was referenced to mean sea level datum. The site reference elevation is the elevation of two nails near the base of a power pole immediately south of the Screen and Grit Building at the WWTP (Figure 2). This elevation is equivalent to 605.72 feet above mean sea level (Reference 2).

3.6 Well Development and Ground-Water Sampling

All monitoring wells were developed by removing several saturated casing volumes of ground water. Well development is necessary to remove the effects of drilling and well installation. A centrifugal pump was used to purge each well. New hoses and fittings were used in each well to prevent cross-contamination between wells during development procedures. Purge water was immediately discharged through a garden hose to the sanitary sewer via a WWTP floor drain.

During development, measurements and observations of pH, specific conductance, temperature, odor, floating product occurrence and turbidity were observed and recorded (Appendix D). When three consecutive samples provided stable readings for these parameters, the well was considered developed. It was estimated that 130, 170 and four gallons (approximately 82, 112, and 3.6 saturated casing volumes) of water was removed from wells MW-1, MW-2, and MW-3, respectively. Well MW-3 could not be developed as thoroughly because of the impeded recharge to the well created by fine grained natural sandpack. A Northern Environmental Technologies

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During well development, a distinct sewage odor and soapy film were observed in the discharged ground water. To determine if sewage impacts existed, a series of routine wastewater parameters was added to the analytical program. These parameters included: biological oxygen demand (BOD₅), chemical oxygen demand (COD), sulfate, chloride, total non-purgeable organic carbon, and fecal coliform.

Three days after development, the wells were sampled. Static water level measurements were recorded prior to sampling. Three saturated casing volumes were purged from each well immediately Three new 1.66 inch diameter stainless steel before sampling. point source bailers were utilized with one bailer dedicated to each well to prevent sample cross-contamination. The wells were sampled by lowering the bailer well below the water surface. This action ensured that the ground-water analyzed would be indicative of products dissolved in the ground water and would not be influenced by the incorporation of floating product layers. Additionally, each bailer was equipped with a teflon bottom device. The bottom emptying device reduces emptying volatilization that normally occurs while decanting the sample from standard bailers. At the conclusion of sampling, each bailer was decontaminated with a methanol/acetone solution rinsed with distilled water, allowed to air dry for 24 hours, and stored. in a new plastic sheath.

Samples collected for metal content analysis were field filtered through a 0.45 micron filter. Field measurements of pH, specific conductance, and temperature were recorded during sampling (Appendix E). Water samples were cooled, packaged and submitted, under chain of custody, for laboratory analysis of the following water quality parameters:

<u>Parameter</u>	Abbreviation	<u>Units</u>
*Biological Oxygen Demand (Five day) *Chemical Oxygen Demand *Sulfate *Chloride *Total Non-Purgeable Organic Carbon Polychlorinated Biphenyls Base, Neutral and Acid Extractable Compounds	BOD ₅ COD SO ₄ C1 TNPOC PCBs	mg/l mg/l mg/l mg/l ug/l
(Semi-volatiles, EPA Method 625) Volatile Organic Compounds Metals: Arsenic Chromium Copper Zinc	BNAS VOCS As Cr Cu Zn	ug/l ug/l mg/l mg/l mg/l
Note: mg/l = milligrams per liter (p ug/l = micrograms per liter (p * = wastewater indicator pa	parts per billion)	

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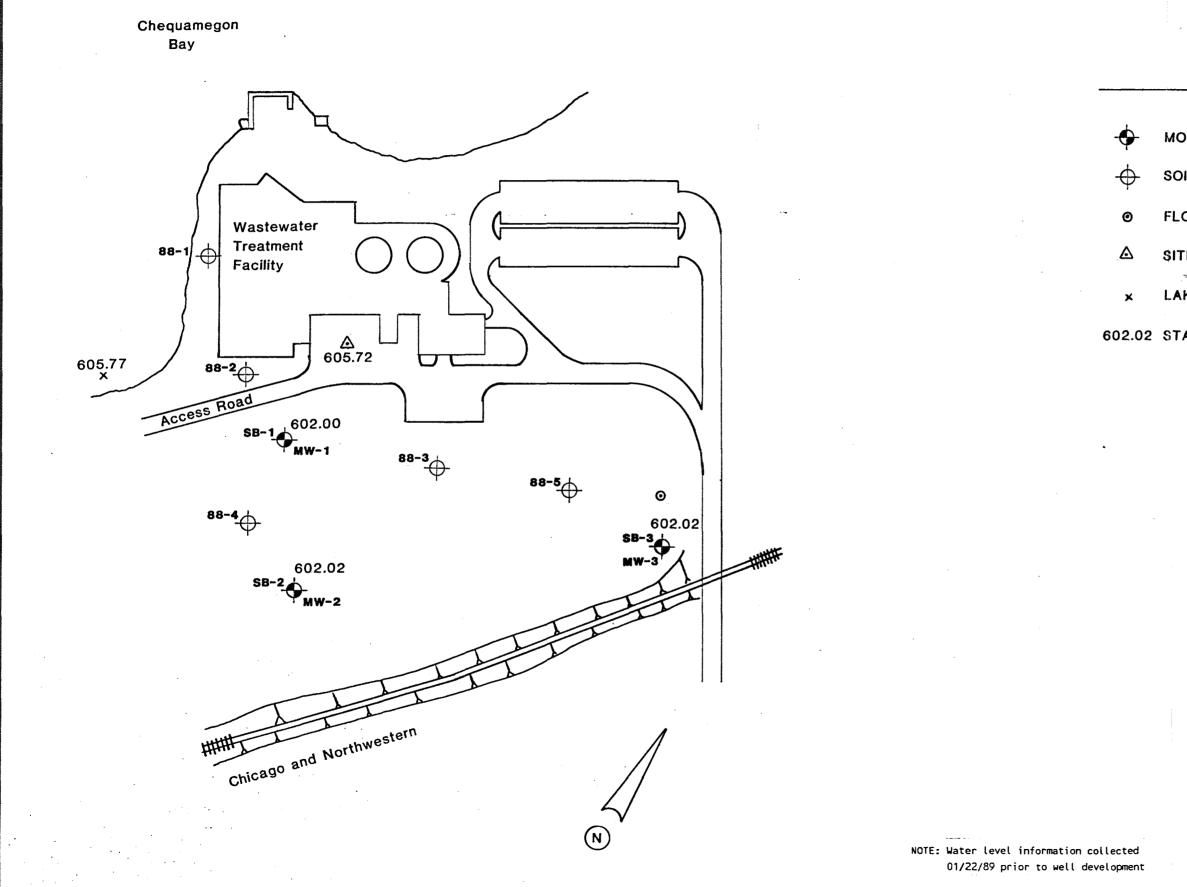
The results of the analyses can be found in Appendix A1.

In addition to the above analyses, fecal coliform content was analyzed by the in-house WWTP laboratory. These lab results are presented in Appendix A2.

3.7 Water Level Measurements and Hydraulic Testing

Water level measurements, referenced to the top of the riser pipe, were made to the nearest 0.01 foot at all monitoring wells. Water level measurements were made with an Olympic water level All water level measurements were converted to site mean probe. sea level datum to allow comparison of water levels. The water carefully level measuring device was rinsed with а methanol/acetone mixture and rinsed with distilled water between subsequent water level measurements. Water level measurement data are shown in Figure 4 and presented in Appendix F.

A series of water level measurements were collected during and after well development to ascertain the hydraulic conductivity of shallow fill materials. The high permeability of these sediments prevented the collection of sufficient data to produce meaningful values of hydraulic conductivity using normal bail recovery test methods and analyses.



14

LEGEND

MONITORING WELL

SOIL BORING

FLOWING ARTESIAN WELL

SITE BENCHMARK

LAKE SUPEPIOR - FROZEN SURFACE

602.02 STATIC WATER LEVEL ELEVATION

0′50′100′150′200′
CITY OF ASHLAND
WASTEWATER TREATMENT PLANT
STATIC
GROUND-WATER ELEVATIONS
PROJECT: BRA10041 DATE: 03/03/89
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4.0 PHYSICAL SETTING

4.1 Site History

Prior to the 1920's, the current WWTP facility site was occupied by the Schrader Sawmill. The Sawmill was reported to have manufactured railroad ties and timbers for dock construction. The railroad ties were treated in a creosote pit reportedly located to the south of the present WWTP facility. It has not known whether the creosote pit was operated by the Schrader Sawmill or one of the various railroad companies in the area. Discussions with WWTP employees have also revealed that the Lake Superior Power District may have used the area for disposal of PCB containing transformers. However, no documents were located which confirm or disprove that this type of disposal has ever occurred at this location (Reference 1).

After the close of the sawmill and creosoting operations, (approximately 1920), the site was characterized by a WWTP employee as a lowland swamp which may have prevented the area from being developed for a number of years. In 1951, the original Ashland Wastewater Treatment Plant was constructed on the lowland site. Since the native soil conditions were unsuitable for standard foundation footings, the facility was built on pilings and concrete support structures. In addition, a clay core wall was constructed along the north and west facility boundaries to impede the inflow of lake water into the sewage treatment process equipment. The location and construction details of the clay core wall are shown in Appendix G.

In 1970, a geotechnical investigation was conducted by Lakehead Testing Laboratory, Incorporated for proposed WWTP additions. Two borings, 70-1 and 70-2, were drilled for the purpose of foundation analysis and were located in the present locations of the final sedimentation tanks and the aeration tanks, respectively. Borehole logs generated by the drilling contractor are included in Appendix B3. Additions to the WWTP were completed in 1973. The WWTP facility has remained basically unchanged to the present day (Reference 1).

4.2 Geology

4.2.1 Regional Geology

Ashland County is located within the Superior lowland of the Superior Region of northern Wisconsin (north of latitude 46° and west of longitude 90°). During the Pleistocene Epoch, the entire region was repeatedly glaciated by ice sheets advancing from Canada. Extensive unconsolidated glacial deposits that were left by the retreat of the last ice sheet now cover the Ashland area to an estimated depth of 100 to 150 feet (Reference 3). ▲ Northern Environmental Technologies Incorporated

The Superior lowland was submerged 9500 - 11500 years ago when widespread lakes stood as much as 500 feet higher than the present Lake Superior. The Superior lowland varies in elevation from 600 to 1080 feet and is characterized by a flat to undulating topography.

The Superior region is covered with glacial material that is represented by two distinct mappable units or formations. These are, from youngest to oldest, the Miller Creek Formation and the In the Ashland area, the Miller Creek Copper Falls Formation. Formation is represented by lake modified glacial sediments. Wave action and deposition during high lake stages produced a subdued topography of low relief. The areal extent of the formation is defined by a low relief, four to 18 mile wide belt that borders the southern shoreline of Lake Superior. Miller Creek till is represented as a reddish unstratified, unsorted sandy silt and clay with scattered pebbles and boulders. Deposition of this Formation occurred between about 11,500 and 9500 years ago (Reference 3).

The older Copper Falls Formation was deposited between 16,000 and 11,500 years ago. The Copper Falls Formation is exposed at the surface south of the Superior lowland and is represented as proglacial stream sediments and sandy till. Stream sediments are characterized by cross-bedded poorly graded sands and gravelly sands. The till is an undifferentiated deposit represented as a reddish brown, unstratified, gravelly, clayey, silty sand. This till is typically overlain by three feet of poorly sorted, gravelly, silty sand. The Copper Falls Formation also underlies the Miller Creek Formation.

The Copper Falls Formation is readily distinguished from the Miller Creek Formation which is somewhat redder and more clayey. Moreover, the Copper Falls Formation contains abundant sand and gravel outwash deposits and stream sediments. Both formations dip or slope gently to the north (Reference 3).

4.2.2 Site Geology

The WWTP site is located on a 500 foot wide terrace between Chequamegon Bay and a retreated shoreline to the south. The terrace was anthropogenically built up over a period of years with clayey fill and wood debris and now lies a few feet above lake level. The fill and sediments on the site have been evaluated and described based on analyses of soil samples collected from eight borings drilled to depths of up to 61 feet.

The entire site is covered with three to 15 feet of silty clay fill and other transported soils. This has been determined from the heterogeneous nature of the material and occurrences of brick rubble and coal cinders. Underlying the clay fill is a layer of wood waste of undetermined lateral extent. The wood waste is composed of saturated shredded wood fibers, chunks, and slabs. ▲ Northern Environmental Technologies In c o r p o r a t e d

Two immiscible phase liquids were encountered in the wood waste zone. Slightly yellow discolored ground water was observed in the void spaces while a dark green oily creosote-like liquid was detected on and in many of the wood fibers. A strong creosote odor was perceived during sample collection and analysis. The average wood waste fill thickness detected across the site was five feet, with a maximum thickness of 9.5 feet in boring SB-1. A minimal (i.e. less than one inch) thickness of wood waste was encountered at boring 88-1. Contour maps are presented that illustrate the depth to top of wood waste, depth to bottom of wood waste, and thickness of wood waste fill (Figures 5, 6, and 7, respectively).

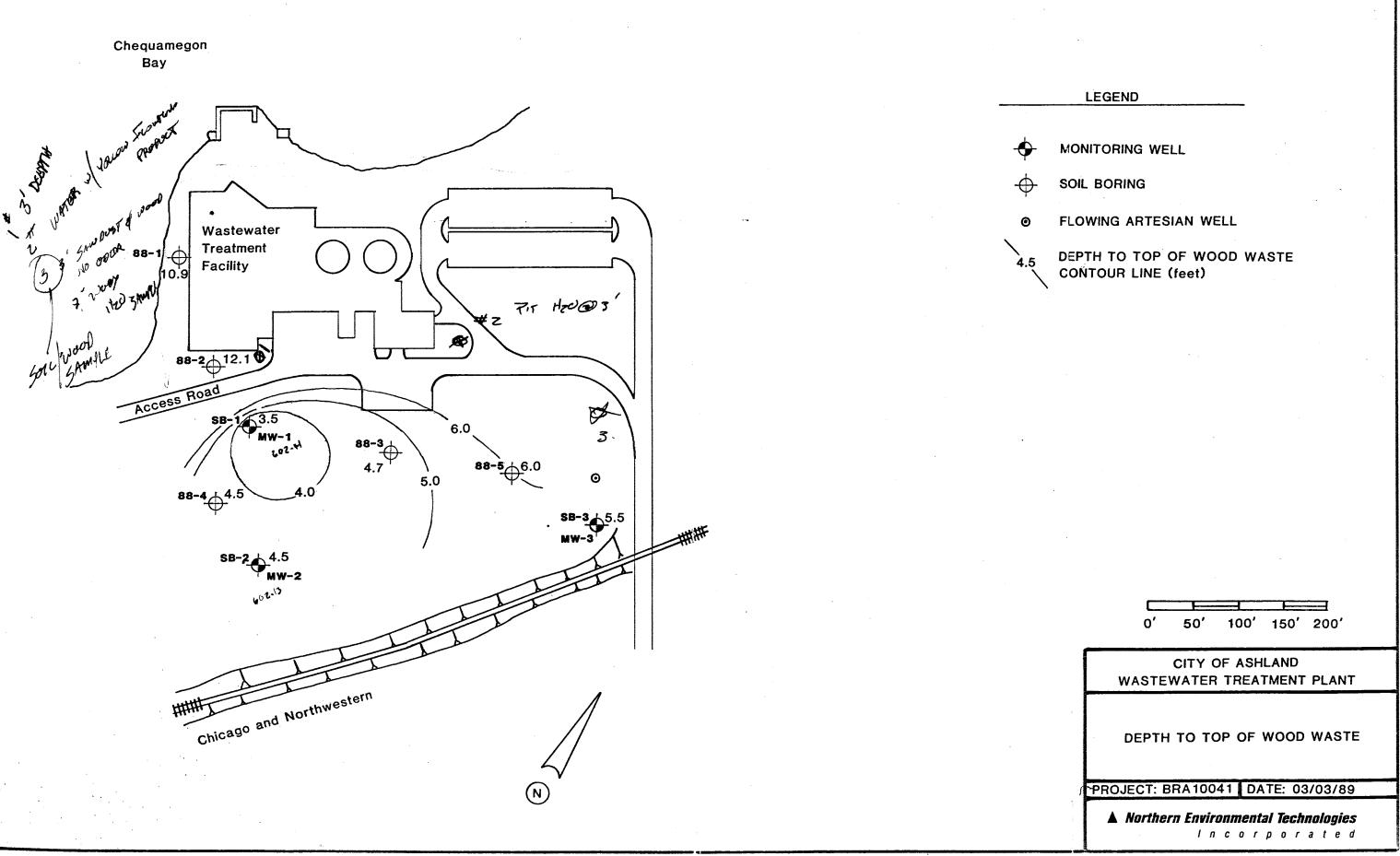
Underlying the wood waste fill are Holocene-age beach deposits and native sediments of the Miller Creek Formation. The Miller Creek Formation on the site consists of a thin discontinuous beach sand unit overlying thick lacustrine clays and glacial till. The upper formational surface is characterized by a thin discontinuous saturated layer of fine to medium, poorly graded sand. This sand unit is zero to five feet thick, and, where present, immediately underlies the wood waste. Below the sand is lake deposited silty clay. The lacustrine unit was observed to be a reddish brown silty clay and sandy silt varying in thickness from nine to 21 feet. Some fine sand seams and a few subrounded pebbles occur throughout this lake deposit.

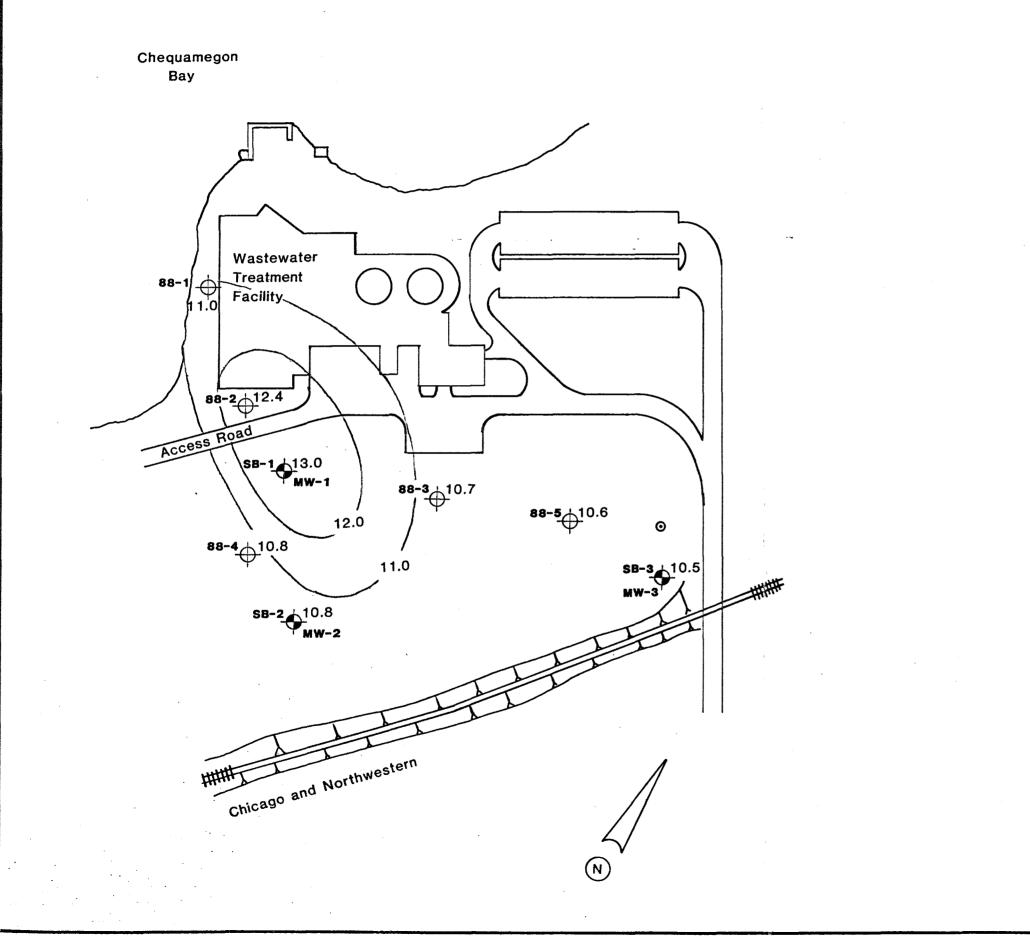
In borings 88-1, 88-2, and 88-3, the lacustrine silty clay is underlain by a discontinuous non-bedded glacial till of the Miller Creek Formation. It varies from a reddish brown gravelly lean clay with sand to a silty sand. Weathered rounded sandstone pebbles up to 3/4-inch in diameter occur sporadically throughout the till. Till thickness across the site varies from 20 feet in boring 88-1 to no occurrence in borings 88-4 and 88-5.

Evidence suggests that the Copper Falls Formation was encountered in the deep borings (88-1 through 88-5) at depths ranging from 25 feet (88-4) to 55 feet (88-1). This is based on the following observations.

- The color of glacial sediments darkened from red to reddish brown with depth.
- Deeper sediments were relatively coarse grained and stratified.
- The deeper sediments contained considerably less clay.

Texturally, the Copper Falls Formation consists of inter-bedded sequences of sand and gravel outwash deposits and glacial till. The lower five to 30 feet of borings 88-1 and 88-3, respectively, were logged as Copper Falls Formation sediments. Neither bedrock, nor the base of the Copper Falls Formation, were encountered during this investigation. A schematic diagram illustrating the site stratigraphy is presented in Figure 8.

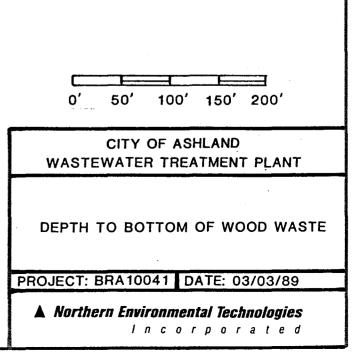


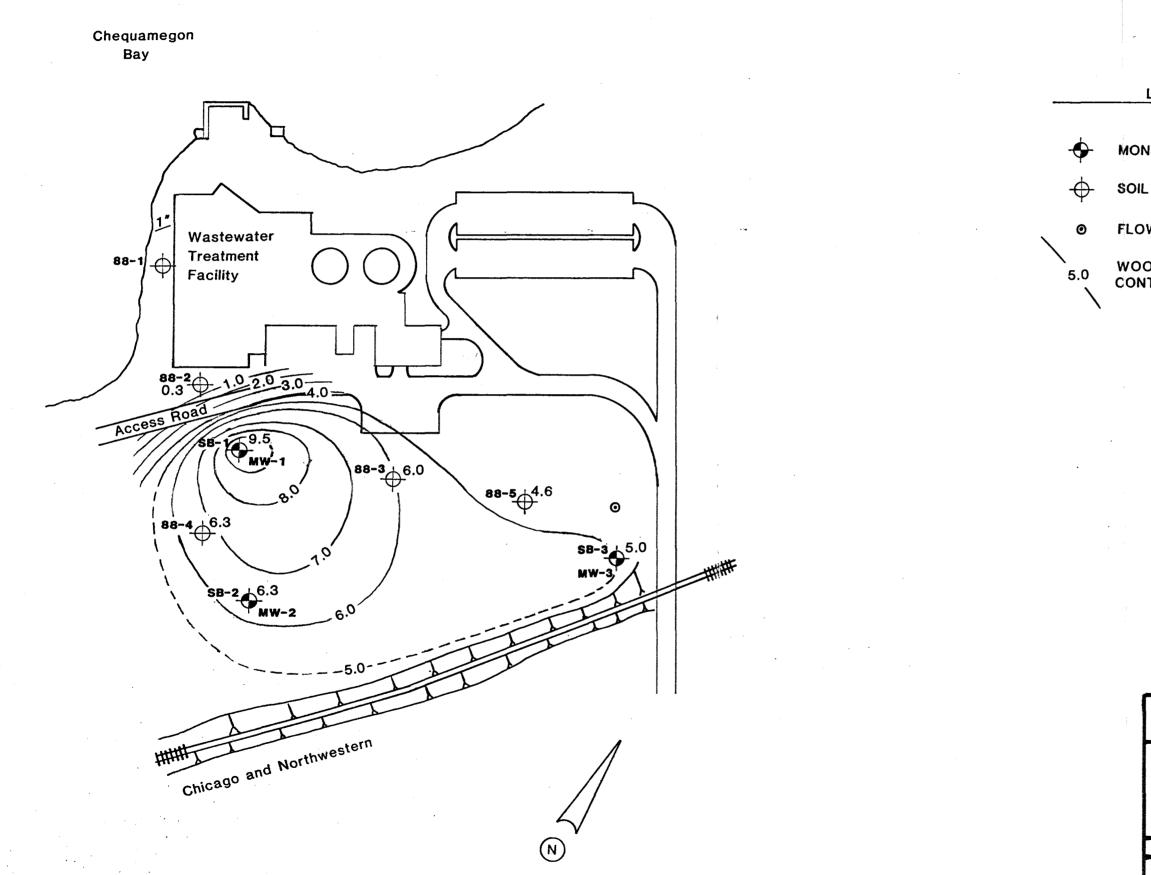


MONITORING WELL SOIL BORING FLOWING ARTESIAN WELL 0 **1**0.7 CONTOUR LINE (feet)

LEGEND

DEPTH TO BOTTOM OF WOOD WASTE





20

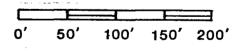
LEGEND

MONITORING WELL

SOIL BORING

FLOWING ARTESIAN WELL

WOOD _WASTE THICKNESS CONTOUR LINE (feet)



CITY OF ASHLAND WASTEWATER TREATMENT PLANT

THICKNESS OF WOOD WASTE

PROJECT: BRA10041 DATE: 03/03/89

▲ Northern Environmental Technologies

FIGURE 7

.

→ Potentiometric Surface

SILTY CLAY, heterogeneous, brown, moist, some

Water <u>⊽</u> Table <u></u>

root matter, trace coal cinders, brick FILL debris, and gravel (3.5 - 14.5 feet thick). FILL WOOD WASTE, yellow-green saturated degraded wood fibers, oily, strong creosote-like odor, wood slabs at base, water table occurs in this zone (0.0 - 9.5 feet thick). LOWER FILL SAND, discontinuous, saturated sand with some silt, poorly graded, where present occurs immediately below wood waste, thin beach BEACH DEPOSIT deposit (0.0 - 0.5 feet thick). SANDY SILTY CLAY, reddish brown, firm, moist, (lake deposited) (9 - 21 feet thick). MILLER LACUSTRINE CREEK FORMATION GRAVELLY CLAY WITH SAND, more coarse than above, contains weathered sandstone pebbles, GLACIAL soft when saturated to very dense silty sand in Boring 88-2, Glacial Till, isolated cobbles or boulders (0 - 20 feet thick). TILL SAND, GRAVEL, AND CLAY, saturated bedded sand and gravel outwash deposits, contains layers of clay till and poorly graded fine, medium, COPPER GLACIAL and coarse sand lenses (greater than 25 FALLS FORMATION OUTWASH feet thick).

UPPER

CITY OF ASHLAND WASTEWATER TREATMENT PLANT

SCHEMATIC OF SITE STRATIGRAPHY

AND

VERTICAL HYDRAULIC GRADIENTS

PROJECT: BRA10041 DATE: 03/03/89

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4.3 Hydrology

4.3.1 Regional Hydrology

Ground water throughout the Lake Superior basin moves naturally from areas of recharge to lower areas of discharge. The ultimate discharge area of ground water in the basin is Lake Superior, where about 100 cubic feet per second discharges to the lake along the Lake Superior shoreline (Reference 4).

Flowing artesian wells are numerous near Lake Superior on the lake plain and around the Bayfield Peninsula. The artesian head results mainly from the restriction of vertical ground-water movement through the thick confining layers of horizontally bedded lake clay. Static heads of more than 30 feet above the level of Lake Superior have been reported near Chequamegon Bay (Reference 4).

Most ground water in the basin is obtained from one of three aquifers:

- 1) Glacial sand and gravel,
- 2) Sandstones, and
- 3) Lava flows.

Sand and gravel aquifers are limited in storage and are normally discontinuous. The suitability of sand and gravel deposits for water supply purposes is entirely dependent on the size and extent of the coarse permeable unit. Sandstone aquifers are extensive and used primarily as a water supply near the City of Superior, Wisconsin, on the fringes of the Bayfield Peninsula, and in or near the City of Ashland. Lava flows are a source of water supply within the lava outcrop area south of the City of Superior, Wisconsin.

4.3.2 Site Hydrology

4.3.2.1 Surface Water

Precipitation which falls onto the site leaves the ground surface as runoff, infiltration, and/or evaporation. All surface water runoff drains immediately north into Lake Superior. No significant source of surface water run-on was observed. Surface water which infiltrates into the soil, escapes evaporation, and percolates to the water table becomes a component of ground-water recharge.

4.3.2.2 Ground Water

Two distinct hydrostratigraphic units were observed within the upper 60 feet of sediments:

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- 1) A shallow water table system existing in the permeable wood waste fill and underlying fine sand lenses, and
- 2) A deep confined glacial outwash aquifer.

These flow systems are separated by a thick sequence (15 to 40 feet) of relatively impermeable lake clay and glacial till.

The shallow ground-water table flow system is primarily confined to the wood waste and hydraulically connected sand lenses. Water level elevations within this water table system system are approximately equal to Lake Superior water level. The lake elevation (as measured to the surface of the ice) was actually three feet higher than the static water levels in wells measured on January 22, 1989 (Figure 4). This was probably due primarily to expansion and build up of ice near the Lake Superior shoreline.

The thickness of the permeable portion of the ground-water table flow system varies from a minimum of two feet in boring 88-2 to 12 feet in boring 88-4. The contour map illustrating wood waste thickness (Figure 7) may be used to approximate the configuration of the permeable flow zone. Recharge to this flow system is primarily by precipitation, lake inflow, and underflow seepage from the shoreline embankment to the south. Discharge of ground water from the water table flow system is primarily by seepage into Lake Superior and by evaporation.

The ground-water table was encountered between four to seven feet below the ground surface across the site. This may be assumed to be representative of normal winter conditions. The water table is extremely flat and nearly horizontal because of the close proximity of Lake Superior and the permeable nature of the wood waste fill. Water level measurements from the recently installed ground-water monitoring wells (MW-1, MW-2, and MW-3) indicate a subtle horizontal hydraulic gradient of 0.0001 foot/foot to the north towards Lake Superior (Figure 4). Due to the extremely low gradient, additional water level data is required to confirm the direction and magnitude of this gradient.

Development of the monitoring wells (Section 3.6) indicates that ground water is readily released from storage within the permeable fill. This is evidenced by high short term well yields. Specific capacities [gallons per minute per foot of drawdown, gpm/ft] were calculated for MW-1 and MW-2 and were found to be in excess of 10 and 40 gpm/ft, respectively. However, sustained yields will probably be less due to the depletion of storage of the limited water table flow system. Applying the method of Hvorslev (Reference 5) to the available water level data indicates that hydraulic conductivities of the wood waste fill are as low as 0.0001 centimeters per second (cm/sec) and range to in excess of 0.005 cm/sec.

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The confined glacial outwash aquifer is found approximately 25 to 55 feet below ground surface. This aquifer is suspected to occur within the relatively permeable sand, gravel, and cobble outwash deposits and stream sediments of the Copper Falls Formation. The unit thickness could not be determined within the scope of this investigation, but is known to exceed 20 feet. Recharge to the outwash aquifer occurs mainly as vertical leakage through the overlying glacial deposits of the Miller Creek Formation and where it is exposed at the surface to the south of the Superior lowland where the lake clays of the Miller Creek Formation are absent.

A strong upward hydraulic gradient of 1.5 foot/foot was calculated between the water table flow system and the deeper outwash aquifer. This value was calculated by comparing the water level elevation at monitoring well MW-3, and the potentiometric head elevation of the flowing artesian well (Figure 8). The potentiometric or artesian head elevation of the artesian well was calculated to be approximately 624 feet mean sea level datum or 17 feet above ground surface. This elevation was calculated from a hydraulic pressure gauge reading of six pounds per square inch (psi), where one psi equals approximately 2.3 feet of hydraulic head.

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5.0 IMPACT INVESTIGATION

5.1 Results of Investigative Activities

5.1.1 Magnetometer Survey

A reconnaissance level magnetometer survey was conducted over the suspected disposal area to assess the presence of buried ferromagnetic objects. Numerous signals were observed which indicated buried ferromagnetic objects throughout the area investigated. The bulk of the signals were due to small, near surface objects. Larger objects were identified in the soil piles south of MW-2 and the immediate area around MW-3. Employees at the WWTP reported to Northern Environmental that the soil piles may contain a few pipes and metal scrap disposed from the treatment plant (Reference 1). No other large anomalous ferromagnetic objects were detected during the magnetometer survey.

5.1.2 Soil Impacts

The results of detailed field investigations and laboratory data indicate that the wood waste fill and soils adjacent to the fill are impacted with a liquid substance resembling creosote (hereafter referred to as creosote). Creosote impacted soils were initially detected during soil boring sample analysis. Impacted samples were generally characterized as oily, dark green, saturated wood chips and fibers.

PID responses were relatively low across the site and within the wood waste zone. However, the PID has a relatively low sensitivity to semivolatile compounds typical of those which constitute creosote. The maximum PID reading on the site (72 ppm) was of sample S3 of the wood waste in boring SB-1 from 4.5 - 6.0 feet (Appendix B1).

Laboratory analysis (Appendix A1) of the composite soil sample DB#2/DB#5 for BNAs strongly suggest that the dominant source of soil impacts is creosote. Soil sample DB#2/DB#5 was composited from boring 88-2 (12.0 - 13.5) and boring 88-5 (0.0 - 10.5 feet). Of the 10 creosote constituents analyzed, nine were detected in DB#2/DB#5 (Table 1). A total of 12 BNA compounds were detected with pyrene [35,000 nanograms per gram (ng/g)], and fluoranthene (15,000 ng/g) detected in the highest concentrations (Table 2). One nanogram per gram (ng/g) is approximately equal to one part; per billion (ppb).

Soil sample DB#4 (boring 88-4, 4.5 - 6.0 feet) was laboratory analyzed for PCBs, VOCs, arsenic, chromium, copper, and zinc. No PCBs or VOCs were detected. However, elevated concentrations of arsenic (100 ug/g), chromium (5 ug/g), copper (46.5 ug/g), and zinc (165 ug/g) were discovered. One microgram per gram (ug/g)

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TABLE 1 - Comparison of Composition of Creosote and Results of Base/Neutral and Acid Extractable Analyses Artesian Well, MW-1, MW-2, MW-3, DB2/DB5 City of Ashland, Wastewater Treatment Plant

						Above	Detect	ion Li	mits
	Perc	ent by I	Weight						
	of	Raw Cree	osote	Analyzed as Part	Artesian				Composite Soil
Component	Α	В	С	of this Study	Well	MW-1	MW-2	MW-3	Sample DB2/DB5
Low Boilers		3.2	-						
Naphthalene	10.5	6.6	3.0	x		x	x	x	x
2-Methylnaphthalene	4.0	8.5	1.2	X			1		
1-Methylnaphthalene	2.15	4.1	0.9						
Biphenyl	1.17		0.8						
Dimethylnaphthalene		4.4	2.0	y					
AcenaphthyLene	0.285			_ X		X	X		X
Acenaphthene.	4.83	5.6	9.0	x		x	x	x	x
Dibenzofuran	3.21	3.9	5.0				2000 Control of the owner of the owner	*************	
Flurorene	3.74	4.2	10.0	x		X	X	X	X
Methylfluorene			3.0						
Phenanthrene	12.0	>11.7	21.0	X		x	X	x	X
'Anthracene	1.56		2.0	(X		x	x	x	x
Carbazole	3.17		2.0	Without concerning the second se	here at	***************************************			*****
Methylphenanthrene			3.0						
Methylanthracene			4.0						
Fluoranthrene	7.32	4.5	10.0	×		<u>, x</u>	x	X	x
Pyrene	5.2	3.4	8.5			(X		X	X
Benzofluorene			2.0						
1,2-Benzanthracene/Chrysene	2.6								
<u>Chrysene</u>		•	3.0	k		X	.	x	<u>x</u>

Total 61.74 60.1 87.7

Sources of Data

A - Mississippi Forest Products Laboratory and Mississippi University, September, 1983.

B - Hepner, R.D., 1977.

C - American Wood Preserver's Association, 1972.

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TABLE 2 - Summary of Laboratory Analysis Results

City of Ashland, Wastewater Treatment Plant

				- -			Composite
	Detection			Ground-Wa	ater Samp	les	Soil Sampl
Parameter	Water (ug/l)	Soil (ng/g)	MW-1 (ug/l)	MW-2 (ug/l)	MW-3 (ug/l)	Artesian Well (ug/l)	DB2/DB5 (ng/g)
SEMI - VOLATILES							
Acenaphthene	20	970	330	300	88	ND	4400
Fluorene	20	970	180	150	83	ND	4900
Phenanthrene	20	4800	370	300	170	ND	11000
Anthracene	20	4800	110	320	100	ND	7100
Fluoranthene	20	970	110	140	260	ND	15000
Pyrene	20	970	230	370	790	ND	35000
Benzo(a)anthracene	20	970	59	93	180	ND	10000
Chrysene	20	970 970	60	93	180	ND	10000
Bis(2-Ethylhexyl)phthalate	20	970 970	18*	73 28	58	ND	ND
Benzo(b)fluoranthrene	20	970 970	53	28 77	150	ND	8000
Benzo(a)pyrene	20	970	51	79	170	ND	8900
Indeno(1,2,3-cd)pyrene	20	970	22	33	80	ND	3500
Dibenz(a,h)anthracene	20	970	ND	14*	26	ND	1400
Benzo(g,h,i)perylene	20	970	27	45	110	ND	4700
Phenol	20	970	ND	14*	ND	ND	ND
Naphthalene	20	970	150	1800	16*	ND	3300
Acenaphthylene	20	970	38	33	ND	ND	1600
VOLATILES							DB4 (ng/g
Benzene	5.0	2.0	190	390	51.4	ND	ND
Ethylbenzene	10.0	4.0	ND	190	ND	ND	ND
Toluene	5.0	2.0	27.1	31.7	ND	ND	ND
m- and p-Xylene	10.0	4.0	73.8	130	ND	ND	ND
o-Xylene	10.0	4.0	150	220	ND	ND	ND
METALS							
Arsenic	5	22000	ND	ND	ND	ND	100000
Chromium	9	2000	ND	ND	ND	ND	5000
Copper	5	1000	23	ND	ND	ND	46500
Zinc	3	600	12	4	16	8	16000

ND = Not Detected

* = Estimated Concentration

ug/l = Parts per billion (ppb)

ng/g = Parts per billion (ppb)

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is approximately equal to one part per million (ppm). Arsenic, chromium, copper, and zinc are metals commonly used in inorganic wood preservative salts.

Dexsil analysis of of sample S6 from boring 88-2 did not detect any PCBs or other chlorinated compounds above ten ppm. Dexsil analysis of auger cuttings collected during drilling at boring SB-1 detected over 50 ppm of PCBs or other chlorinated organic substance. However, no PCBs were detected in any of the soil samples submitted for laboratory analysis.

The vertical extent of soil impacts appears to be limited to the soils adjacent to the wood waste and the underlying discontinuous thin sand lenses. This is supported by visual inspection and low PID readings on the deeper lacustrine clays and glacial tills. Furthermore, none of the BNAs detected in the wood waste were detected in the artesian well which is completed in the lower confined aquifer.

Impacted soils underlie at least 1.1 acres of WWTP property south of the treatment facility. All the borings, except 88-1, penetrated significant thicknesses of impacted soils and wood waste. Therefore, the total lateral extent of soil impacts could not be fully defined in this investigation. In view of the physiography of the shoreline, it is expected that the wood waste disposal area may exist further to the northeast and southwest, paralleling the shoreline of Chequamegon Bay.

Cleanup guidelines for VOC contaminated soils have been established by the Wisconsin Department of Natural Resources (WDNR) at 10 ppm in Reference 6. This limit represents the practical limit of detection, for VOCs in soils, achievable in field and laboratory analysis. No VOC soil impacts were detected.

5.1.3 Ground-Water Impacts

The water table flow system is impacted with the dissolved constituents of creosote and four priority pollutant VOCs. Table 2 summarizes compounds detected as part of this study. Of the 10 BNA constituents of creosote analyzed, all were detected in ground-water monitoring wells MW-1 and MW-2. Ground water from monitoring well MW-3 contained nine of the BNA constituents of creosote above the detection limit (Table 1). Seventeen BNA compounds were consistently detected in all the monitoring wells which indicated shallow ground-water impacts exist across the The compounds detected in the highest concentrations site. included naphthalene (1800 ug/l) in MW-2 and pyrene (790 ug/l) in Of the VOCs analyzed, benzene, ethylbenzene, toluene, and MW-3. xylene (BETX were detected (Appendix A1). These compounds are typical components of light grade petroleum fuels (e.g. kerosene and diesel). It was common practice to cut creosote with kerosene and/or diesel during the wood preserving process. No

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PCBs were detected in ground water collected from any of the wells.

In addition to dissolved impacts, all of the ground-water monitoring wells installed as part of this study appeared to contain a floating immiscible product phase. This immiscible product phase consisted of a yellowish or greenish yellow oily substance. This floating phase was estimated to be no more than one inch thick.

A series of routine wastewater parameters were also analyzed to determine if sewage impacts existed in the water table zone. Sewage impacts were suspected during well development when characteristic sewage odors and soapy films were observed in the A water sample was collected for fecal pumped ground water. coliform analysis from the three monitoring wells and analyzed by the WWTP in-house laboratory. No fecal coliform populations were detected (Appendix A2). COD values were at least one order of magnitude higher than BOD values (Appendix A1). The water sample from MW-3 exhibited a COD value of 1870 mg/l which indicates significant uptake of oxygen by organic and inorganic matter. These data indicate that no noticeable sewage impacts exist within the water table flow system and offer additional supporting evidence that on-site creosote is the source of identified impacts.

The vertical extent of impacted ground-water appears to be confined to the water table system. This is supported by the following observations:

- No soil impacts were observed in deeper soils during visual inspection and PID screening of soil samples collected from the geotechnical borings.
- The relatively impermeable lacustrine clay and glacial till units of the Miller Creek Formation are up to 20 thick and would severely limit vertical migration of contaminants.
- There is a strong upward vertical hydraulic gradient (estimated to be 1.5 feet/feet) between the lower confined outwash aquifer and the water table system. The upward flow of ground water would prevent the downward migration of contaminants.
- No BNAs or other organic parameters were detected in the sample collected from the artesian well.

The lateral extent of ground-water impacts within the water table flow system could not be fully defined given the available information. The lateral extent of ground-water impacts will depend upon the size of the wood waste disposal area, the permeability of the fill material, and the lateral extent of the discontinuous sand lenses that occur at the base of the fill.

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Ground-water quality standards are established by the WDNR in Reference 7. Ground-water sample analyses from monitoring wells MW-1, MW-2, and MW-3 were compared with WDNR ground-water quality standards (Table 3). Two parameters exceed the water quality standards. The WDNR enforcement standard (ES) for benzene (0.67 ppb) was exceeded for all wells (MW-1: 190 ppb, MW-2: 39 ppb, and MW-3: 350 ppb). The WDNR preventative action limit (PAL) for xylene (124 ppb) was exceeded in two wells (MW-1: 223.8 ppb and MW-2: 350 ppb). The violation of these indicator parameters have been verbally reported to the WDNR per the requirements of s. NR140.24 and NR140.26 (Reference 8). This report meets the written requirements of these sections of the Wisconsin Administrative Code.

5.2 Migration Potential and Fate

The migration potential of creosote and BETX impacted ground water on and within the water table flow system is expected to be minimal. This is based on the following observations.

- Horizontal hydraulic gradients at the site are extremely low (0.0001 foot/foot).
- Thick, relatively impermeable deposits of lacustrine clay and glacial till underlying the water table flow system coupled with strong upward vertical hydraulic gradient should prevent downward migration of contaminants.
- Although disposal of creosote compounds probably ceased over 50 years ago, significant concentrations of wood preservative compounds remain on the WWTP site.
- The majority of the compounds which compose creosote are not soluble in water.
- The periphery of the wood waste disposal area is most likely "pinched out" between the upper clay fill and the lower lacustrine clays limiting lateral migration of impacted ground water in the relatively permeable wood waste.

A potential migration route exists within underlying sand lenses, whereby contaminants could be transported off site under the low lateral hydraulic gradient. The continuity of the thin sand deposits will have a direct influence on the significance of the potential migration pathway. The most likely fate of impacted ground water is seepage into Lake Superior. This potential has been inadvertently minimized by the construction of a clay core wall. A clay core wall was constructed in 1952 along the north and western borders of the initial treatment plant to impede subsurface water inflow from Lake Superior to the WWTP area (Appendix G). Incorporated

TABLE 3 - Comparison of Monitoring Well Water Sample Analysis Results with WDNR Public Health Ground-Water Quality Standards City of Ashland, Wastewater Treatment Plant

Parameter	WDNR Preventative Action Limit (PAL) (ug/l)	WDNR Enforcement Standard (ES) (ug/l)	Detection Limit (ug/l)	MW-1 (ug/l)	MW-2 (ug/l)	MW-3 (ug/l)	Artesian Well (ug/l)
METALS						-	
Arsenic	5	50	5.0	ND	ND	ND	ND
Chromium	5	50	9.0	ND	ND	ND	ND
VOLATILES		<u>,</u>					
Benzene	0.067	0.67	5.0	190+	390+	51.4+	ND
1,2-Dichlorobenzene	125	1250	10.0	ND	ND	ND	ND
1,3-Dichlorobenzene	125	1250	10.0	ND	ND	ND	ND
1,4-Dichlorobenzene	150	750	10.0	ND	ND	ND	ND
1,1-Dichloroethane	85	850	5.0	ND	ND	ND	ND
1,2-Dichloroethane	0.05	0.5	5.0	ND	ND	ND	ND
1,1-Dichloroethylene	0.024	0.24	10.0	ND	ND	ND	ND
1,2-Dichloroethylene	10	100	10.0	ND	ND	ND	ND
Ethylbenzene	272	1360	10.0	ND	190	ND	ND
Tetrachloroethylene	1.0	0.1	5.0	ND	ND	ND	ND
Toluene	68.6	343	5.0	27.1	31.7	ND	ND
1,1,1-Trichloroethane	40	200	5.0	ND	ND	ND	ND
1,1,2-Trichloroethane	0.06	0.6	5.0	ND	ND	ND	ND
Trichloroethylene	1.8	0.18	5.0	ND	ND	ND	ND
Vinyl Chloride	0.015	0.0015	20.0	ND	ND	ND	ND
Trichlorofluoromethane	3500	700	10.0	ND	ND	ND	ND
Xylene	124	620	10.0	223.8*	350*	ND	ND

ND = Not Detected

+ = WDNR Enforcement Standard Exceeded

* = WDNR Preventative Action Limit Exceeded

ug/l = Parts per billion (ppb)

▲ Northern Environmental Technologies Incorporated

5.3 Environmental and Human Health and Welfare Impacts

There does not appear to be an immediate environmental or public health risk associated with the identified creosote and VOC impacts at the WWTP facility. No residential water supply wells were identified down gradient. Discharge of impacted ground water would probably occur along the Lake Superior shoreline. Dilution of contaminants to background levels would be expected in Lake Superior long before reaching the City of Ashland water intakes.

The most probable route of entry to individuals would be by dermal contact. Should the waste be exposed at the surface (e.g. as during excavation and construction activities), prolonged contact with the skin can cause burns or rashes to develop. When ingested internally, creosote is a suspected carcinogen. To date, there have been no abnormal health effects, complaints, or incidents filed that have been associated with the creosote disposal pit.

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▲ Northern Environmental Technologies Incorporated

6.0 CONCLUSIONS

Nine creosote containing compounds and elevated concentrations of arsenic, chromium, copper, and zinc were detected in soil samples collected from the wood waste layer. Laboratory analysis of samples collected from ground-water quality monitoring wells on the site indicates that dissolved components of creosote exist in the ground water (Table 2). Although, the concentrations of these dissolved components do not appear to currently pose a threat to human health and welfare or the environment, the concentrations of benzene detected exceeds the WDNR Enforcement Standard and the concentration of xylene detected exceeds the WDNR Preventative Action Limit. ▲ Northern Environmental Technologies In corporated

7.0 REFERENCES

- Conversation: J. Dellaport (Northern Environmental) with D. Wasepka (City of Ashland) and other WWTP employees, January 16 through February 8, 1989.
- Conversation: J. Dellaport (northern Environmental) with M. Siep (Bonestroo, Rosene, Anderlik, & Associates), January 30, 1989.
- 3) Clayton, Lee, "Pleistocene Geology of the Superior Region, Wisconsin", Geological and Natural History Survey, Information Circular Number 46, 1984.
- 4) Young, H.L. and Skimner, E.L., "Water Resources of Wisconsin Lake Superior Basin ", U.S. Geological Survey, Hydrologic Investigations Atlas HA-524.
- 5) Hvorslev, M.J., "Time Log and Soil Permeability in Ground-Water Observations", U.S. Army Corps of Engineers, Waterways Experiment Station, Vicksburg, Mississippi, Bulletin Number 36, 1951.
- 6) Letter: P. Didier (WDNR) to District Directors (WDNR),
 "Practices and Standards for the Management of VOC -Contaminated Soils", April 18, 1989.
- 7) s. NR 140, Wis. Adm. Code, "Groundwater Quality", October, 1988.
- 8) Conversation: D. Rautmann (Northern Environmental) with D. Kafura (WDNR), January 24, 1989.

▲ Northern Environmental Technologies Incorporated

APPENDIX A

RESULTS OF CHEMICAL ANALYSIS

*

▲ Northern Environmental Technologies Incorporated

APPENDIX A1

ENVIROSCAN LABORATORY REPORTS

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February 16, 1989

Northern Environmental Technologies P.O. Box 532 Cedarburg, WI 53012

Attn: John Dellaport

Re: Project BRA 10041 Ashland Site Assessment

Attached are the analytical results for the samples received January 27, 1989. All analyses were done in accordance with EPA Methods (EPA-600/4-79-020).

If you have any questions about the results, please call.

Sincerely,

ENVIROSCAN, INC.

ines R. Salkowski

James R. Salkowski Manager of Inorganic Laboratories

JRS/1s

303 West Military Road Rothschild, WI 54474 (715) 359-7226

ENVIRESCAN

February 13, 1989

Northern Environmental W61 N313 Washington Ave. P.O. Box 532 Cedarburg, WI 53012

Attn: John Dellaport

Re: Ashland Site

Attached are the results of the samples received on January 30, 1989. The analyses were all done in accordance to EPA methods. The soil sample results are reported on a dry weight basis.

The chain of custody documents are also attached.

If you have any questions, please call.

Sincerely,

ENVIROSCAN, INC.

Schanen

Gary L. Scharrer Analytical Chemist

GLS/1s

303 West Military Road Rothschild, W1 54474 (715) 359-7226

FOR:

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Northern Environmental W61 N313 Washington Ave. P.O. Box 532 Cedarburg, WI 53012

Attn: John Dellaport

Re: Ashland Site

NALYII (CAL REPORT

WERONALINUAL AND ANALYTICAL SERVICES WISCONSIN LAB CERTIFICATION NO. 737053130

P.O. #:BRA	10041	
SAMPLED BY:	Client	
DATE REC'D:	1-30-89	
REPORT DATE:	2-1-3-89	
APPROVED BY:	A.S.	

Compound	Supply Well	<u>MW-1</u>	<u>MW-2</u>	<u>MW-3</u>	Detection Limit
Aroclor 1016	X	x	x	X	0.2
Aroclor 1221	x	x	x	X	0.2
Aroclor 1232	x	x	x	. X	0.2
Aroclor 1242	x	x	х	х	0.2
Aroclor 1248	х	x	x	х	0.2
Aroclor 1254	x	x	x	х	0.2
Aroclor 1260	х	х	x	х	0.2
Analytical No.	12914	12915	12916	12917	
X = Analyzed but	not dete	cted			

PCB Analysis (ug/l)

A DYDICAL REPORT

FOR:



SON VENTAL AND ANALYTIC RESERVICES WISCONSIN LAB CERTIFICATION NO. 737053130

P.O. #:	BRA	10014
SAMPLE	DBY:	Client
DATE RE	C'D:	1-30-89
REPORT	DATE:_	2-13-89
APPROVE	ED BY:	

Northern Environmental W61 N313 Washington Ave. P.O. Box 532 Cedarburg, WI 53012

Attn: John Dellaport

Re: Ashland Site

PCB Analysis (ng/g)

Compound	DB-4	Detection Limit*
Aroclor 1016	x	35.0
Aroclor 1221	x	35.0
Aroclor 1232	x	35.0
Aroclor 1242	x	35.0
Aroclor 1248	x	35.0
Aroclor 1254	x	35.0
Aroclor 1260	x	35.0
Analytical No.	12918	
X = Analyzed but not of		

* Calculated on dry weight basis

NAUGYER (CALL REPORT



FOR:

Northern Environmental W61 N313 Washington Ave. P.O. Box 532 Cedarburg, WI 53012

Attn: John Dellaport

Re: Ashland Site

ENVERONMENTAL AND ANALA TICAL SERVICE WISCONSIN LAB CERTIFICATION NO. 737053130

P.O. #:	_
SAMPLED BY: C	lient
DATE BEC'D: 1	-30-89
REPORT DATE: 2	-13-89
APPROVED BY:	St.

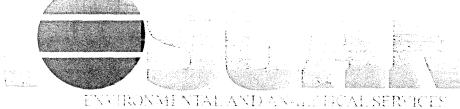
EPA 625 Analysis (ug/l)

	Supply		Detection
Compound	Well	<u>MW-1</u>	Limit
Phenol	х	X	20.
bis(2-Chloroethyl)ether	Х	X	20.
2-Chlorophenol	Х	X	20.
1,3-Dichlorobenzene	Х	X	20.
1,4-Dichlorobenzene	х	X	20.
1,2-Dichlorobenzene	Х	X	20.
bis(2-Chloroisopropyl)ether	X	X	20.
N-Nitroso-di-n-propylamine	Х	Х	20.
Hexachloroethane	Х	Х	20.
Nitrobenzene	X	Х	20.
Isophorone	х	Х	20.
2-Nitrophenol	Х	Х	20.
2,4-Dimethylphenol	Х	Х	20.
bis(2-Chloroethoxy)methane	X	Х	20.
2,4-Dichlorophenol	X	X "	20.
1,2,4-Trichlorobenzene	×	Х	20.
Naphthalene	Х	150.	20.
Hexachlorobutadiene	Х	Х	20.
4-Chloro-3-methylphenol	Х	X	20.
2-Methylnaphthalene	Х	Х	20.
Hexachlorocyclopentadiene	Х	Х	20.
2,4,6-Trichlorophenol	Х	Х	20.
2-Chloronaphthalene	Х	Х	20.
Dimethylphthalate	Х	Х	20.
Acenaphthylene	Х	38.	20.
2,6-Dinitrotoluene	Х	Х	20.
Benzidine	X	X	100.
Analytical No.	12914	12915	

X = Analyzed but not detected

NALYTICAL REPORT

FOR:



WISCONSIN LAB CERTIFICATION NO. 737053130

P.O. #:	
SAMPLED BY:	Client
DATE REC'D:	1-30-89
REPORT DATE:	2-13-89
APPROVED BY:	All.

Northern Environmental W61 N313 Washington Ave. P.O. Box 532 Cedarburg, WI 53012

Attn: John Dellaport

Re: Ashland Site

EPA 625 Analysis (ug/l)

Compound	Supply Well	<u>MW-1</u>	Detection Limit	
Acenaphthene	Х	330.	20.	
2,4-Dinitrophenol	Х	Х	100.	
4-Nitrophenol	Х	Х	100.	
2,4-Dinitrotoluene	Х	Х	20.	
Diethylphthalate	X	Х	20.	
4-Chlorophenyl-phenylether	X	Х	20.	
14 Fluorene	Х	180.	20.	
4,6-Dinitro-2-methylphenol	X	Х	100.	
N-Nitrosodiphenylamine(1)	X	Х	20.	
4-Bromophenyl-phenylether	X	Х	20.	
Hexachlorobenzene	X	Х	20.	
Pentachlorophenol	X	X	100.	
J ⁵ Phenanthrene	X	370.	20.	
5 Anthracene	<u> </u>	110.	20.	
Di-n-butylphthalate	4.(J)	X	20.	
Fluoranthene	X	110.	20.	
Pyrene	Х	230.	20.	
Butylbenzylphthalate	Х	X	20.	
3,3'-Dichlorobenzidine	Х	X	40.	
Benzo(a) anthracene	X	59.	20.	
Chrysene	Х	60.	20.	
bis(2-Ethylhexyl)phthalate	Х	18.(J)	20.	
Di-n-octylphthalate	Х	Х	20.	
Benzo(b)fluoranthene	Х	53.	20.	2
Benzo(k)fluoranthene	Х	X	20.	
4 Benzo(a) pyrene	Х	51.	20.	
Indeno(1,2,3-cd)pyrene	Х	22.	20.	
,Dibenz(a,h)anthracene	Х	Х	20.	
<pre>%Benzo(g,h,i)perylene</pre>	X	27.	20.	
Analytical No.	12914	12915		

X = Analyzed but not detected
J = Estimated Conc.

TAYN CAL RIDPORT

FOR:

ENVIRONMENTAL AND ANALOTTICAL WISCONSIN LAB CERTIFICATION NO. 737053130

SAMPLED BY: Client	
DATE REC'D: 1-30-89	
REPORT DATE: 2-13-89	
APPROVED BY:	

Northern Environmental W61 N313 Washington Ave. P.O. Box 532 Cedarburg, WI 53012

John Dellaport Attn:

Re: Ashland Site

EPA 625 Analysis (ug/l)

Compound	<u>MW-2</u>	<u>MW-3</u>	Detection Limit	
2 Phenol	1 4. (J)	x	20.	
bis(2-Chloroethyl)ether	X	Х	20.	
2-Chlorophenol	Х	Х	20.	
1,3-Dichlorobenzene	X	Х	20.	
1,4-Dichlorobenzene	Х	Х	20.	'
1,2-Dichlorobenzene	X	Х	20.	
bis(2-Chloroisopropyl)ether	Х	Х	20.	
N-Nitroso-di-n-propylamine	X	Х	20.	·
Hexachloroethane	X	Х	20.	
Nitrobenzene	Х	Х	20.	•
Isophorone	Х	Х	20.	
2-Nitrophenol	Х	Х	20.	
2,4-Dimethylphenol	Х	Х	20.	
bis(2-Chloroethoxy)methane	X	Х	20.	
2,4-Dichlorophenol	́Х Х	X	20.	
1,2,4-Trichlorobenzene	X	Х	20.	
9Naphthalene	1800.	16.(J)	20.	
Hexachlorobutadiene	X	X	20.	
4-Chloro-3-methylphenol	Х	Х	20.	
2-Methylnaphthalene	Х	X	20.	
Hexachlorocyclopentadiene	Х	X	20.	
2,4,6-Trichlorophenol	Х	Х	20.	
2-Chloronaphthalene	Х	Х	20.	
Dimethylphthalate	X	Х	20.	
Acenaphthylene	33.	Х	20.	ì
2,6-Dinitrotoluene	Х	X	20.	
Benzidine	x	X	100.	
Analytical No.	12916	12917		

X = Analyzed but not detected J = Estimated Conc.



WISCONSIN LAB CERTIFICATION NO. 737053130

P.O. #:	
SAMPLED BY:	Client
DATE REC'D:	1-30-89
REPORT DATE:	2-13-89
APPROVED BY:	XII.

Northern Environmental W61 N313 Washington Ave. P.O. Box 532 Cedarburg, WI 53012

Attn: John Dellaport

Ashland Site Re:

EPA 625 Analysis (ug/l)

			Detection
Compound	MW-2	<u>MW-3</u>	Limit
9 Acenaphthene	300.	88.	20.
2,4-Dinitrophenol	X	X	100.
4-Nitrophenol	X	X	100.
2,4-Dinitrotoluene	X	X	20.
Diethylphthalate	х	Х	20.
4-Chlorophenyl-phenylether	x	X	20.
18 Fluorene	150.	83.	20.
4,6-Dinitro-2-methylphenol	Х	Х	100.
N-Nitrosodiphenylamine(1)	Х	Х	20.
4-Bromophenyl-phenylether	Х	Х	20.
Hexachlorobenzene	х	Х	20.
Pentachlorophenol	х	Х	100.
} S Phenanthrene	300.	170.	20.
SAnthracene	320.	100.	20.
Di-n-butylphthalate	× X	X	20.
Fluoranthene	140.	260.	20.
Pyrene	370.	790.	20.
Butylbenzylphthalate	Х	Х	20.
3,3 [°] -Dichlorobenzidine	Х	X	40.
Benzo(a)anthracene	93.	180.	20.
Chrysene	92.	180.	20.
\ ¹ bis(2-Ethylhexyl)phthalate	28.	58.	20.
Di-n-octylphthalate	X	Х	20.
Benzo(b)fluoranthene	77.	150.	20.
Benzo(k)fluoranthene	Х	Х	20.
Benzo(a)pyrene	79.	170.	20.
Indeno(1,2,3-cd)pyrene	33.	80.	20.
Dibenz(a,h)anthracene	14.(J)	26.	20.
¦∦Benzo(g,h,i)perylene	45.	110.	20.
Analytical No.	12916	12917	

X = Analyzed but not detected J = Estimated Conc.

FOR:

FOR:

ENVIRONMENTAL AND ABOLEVER CAL SERVICES WISCONSIN LAB CERTIFICATION NO. 737053130

NALYTICAL REPORT

Northern Environmental Technologies	P.O. #:
W61 N313 Washington Ave.	SAMPLED BY: Client
P.O. Box 532	DATE REC'D: 1-30-89
Cedarburg, WI 53012	REPORT DATE: 2-24-89
	APPROVED BY:

Attn: John Dellaport

Re: Ashland Site

Compound	DB-2/DB-5 Composite	Detection Limit*
Phenol	X	970.
bis(2-Chloroethyl)ether	X	970.
2-Chlorophenol	Х	970.
1,3-Dichlorobenzene	Х	970.
1,4-Dichlorobenzene	Х	970.
1,2-Dichlorobenzene	Х	970.
bis(2-Chloroisopropyl)ether	Х	970.
N-Nitroso-di-n-propylamine	Х	970.
Hexachloroethane	Х	970.
Nitrobenzene	Х	970.
Isophorone	Х	970.
2-Nitrophenol	Х	970.
2,4-Dimethylphenol	Х	970.
bis(2-Chloroethoxy)methane	Х	970.
2,4-Dichlorophenol	Х	970.
1,2,4-Trichlorobenzene	X	970.
9 Naphthalene	3300.	970.
Hexachlorobutadiene	Х	970.
4-Chloro-3-methylphenol	Х	970.
2-Methylnaphthalene	X	970.
Hexachlorocyclopentadiene	х	970.
2,4,6-Trichlorophenol	Х	970.
2-Chloronaphthalene	X	970.
Dimethylphthalate	X	970.
Acenaphthylene	Х	970.
2,6-Dinitrotoluene	1600.	970.
Benzidine	х	4800.

EPA 625 Analysis (ng/g)

Analytical No.

12918

X = Analyzed but not detected

* Calculated on dry weight basis

FOR:

N TRONMENTAL MENTROAL SERVICE .

WISCONSIN LAB CERTIFICATION NO. 737053130

Northern Environmental Technologies	P.O. #:
W61 N313 Washington Ave.	SAMPLED BY
P.O. Box 532	DATE REC'D:
Cedarburg, WI 53012	REPORT DATI
	APPROVED B

P.O. #:		
SAMPLED BY:	Client	
DATE REC'D:	1-30-89	
REPORT DATE:	2-24-89	
APPROVED BY:	M.	

Attn: John Dellaport

Ashland Site Re:

N (CAN LI REL 20)

EPA 625 Analysis (ng/g)

Compound	DB-2/DB-5 Composite	Detection Limit*
Acenaphthene	4400.	970.
2,4-Dinitrophenol	X	4800.
4-Nitrophenol	х	4800.
2,4-Dinitrotoluene	х	970.
Diethylphthalate	X	970.
4-Chlorophenyl-phenylether	Х	970.
⁴ Fluorene	4900.	970.
4,6-Dinitro-2-methylphenol	Х	4800.
N-Nitrosodiphenylamine(1)	X	970.
4-Bromophenyl-phenylether	X	970.
Hexachlorobenzene	X	970.
Pentachlorophenol	Х	4800.
15 Phenanthrene	11000.	4800.
Anthracene	7100.	970.
Di-n-butylphthalate	X	970.
Fluoranthene	15000.	970.
Pyrene	* 35000.	970.
Butylbenzylphthalate	X	970.
3,3'-Dichlorobenzidine	X	1900.
Benzo(a) anthracene	10000.	970.
Chrysene	10000.	970.
<pre>bis(2-Ethylhexyl)phthalate</pre>	X	970.
Di-n-octylphthalate	Х	970.
Benzo(b)fluoranthene	8000.	970.
Benzo(k) fluoranthene	X	970.
(&Benzo(a)pyrene	8900.	970.
Indeno(1,2,3-cd)pyrene	3500.	970.
Dibenz(a,h)anthracene	1400.	970.
Benzo(g,h,i)perylene	4700.	970.
	10010	

Analytical No.

12918

X = Analyzed but not detected * Calculated on dry weight basis

NALYTICAL REPORT



FOR:

Northern Environmental W61 N313 Washington Ave. P.O. Box 532 Cedarburg, WI 53012 ENVIRON MENTAL AND ANALYTICAL SERVIC WISCONSIN LAB CERTIFICATION NO. 737053130

P.O. #:	BRA	10041
SAMPLED	BY:	Client
DATE REC	'D:	1-30-89
REPORT D	ATE:	2-13-89
APPROVE	DBY: (Judy Squandowski

VOC Analysis (ug/l)

	Detection Limit	Supply Well
Benzene	0.5	х
Bromoform	2.0	X
Bromomethane	4.0	X
Carbon Tetrachloride	0.5	Х
Chlorobenzene	2.0	Х
Chloroethane	2.0	X
2-Chloroethylvinyl Ether	5.0	Х
Chloroform	0.5	Х
Chloromethane	2.0	Х
Dibromochloromethane	0.5	Х
1,2-Dichlorobenzene	1.0	Х
1,3-Dichlorobenzene	1.0	Х
1,4-Dichlorobenzene	1.0	Х
Dichlorobromomethane	0.5	Х
l,1-Dichloroethane	0.5	Х
1,2-Dichloroethane	0.5	Х
l,l-Dichloroethylene	1.0	Х
1,2-Dichloroethylene	1.0	Х
Dichloromethane	1.0	Х
1,2-Dichloropropane	0.5	Х
cis-1,3-Dichloropropene	2.0	Х
trans-1,3-Dichloropropene	0.5	Х
Ethylbenzene	1.0	Х
1,1,2,2-Tetrachloroethane	1.0	Х
Tetrachloroethylene	0.5	X
Toluene	0.5	, X
1,1,1-Trichloroethane	0.5	X
1,1,2-Trichloroethane	0.5	X
Trichloroethylene	0.5	X
Vinyl Chloride	2.0	X
Trichlorofluoromethane	1.0	X
Dichlorodifluoromethane	2.0	Х
m & p-Xylene	1.0	X
o-Xylene	1.0	X

12914

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X = Analyzed but not detected

Analytical No.

LYTICAL REPO

FOR:

ENGIRONMENTA: AND ANALY FRALSERVELS WISCONSIN LAB CERTIFICATION NO. 737053130

Northern Environmental P.O. #: _____BRA_10014 W61 N313 Washington Ave. Client SAMPLED BY: P.O. Box 532 1-30-89 DATE REC'D: ____ Cedarburg, WI 53012 2-13-89 REPORT DATE:__ APPROVED BY: Quely Sewandouter

. .

VOC Analysis (ug/l)

	Detection			
	Limit	MW-1	MW-2	MW-3
£\$				
5 VBenzene	5.0	190.	390.	51.4
Bromotorm	20.0	Х	X	X
Bromomethane	40.0	Х	Х	Х
Carbon Tetrachloride	5.0 ·	Х	X	Х
Chlorobenzene	20.0	Х	Х	Х
Chloroethane	20.0	Х	Х	Х
2-Chloroethylvinyl Ether	50.0	Х	Х	Х
Chloroform	5.0	Х	Х	Х
Chloromethane	20.0	Х	Х	Х
Dibromochloromethane	5.0	Х	Х	Х
1,2-Dichlorobenzene	10.0	Х	Х	Х
1,3-Dichlorobenzene	10.0	Х	Х	X
1,4-Dichlorobenzene	10.0	Х	Х	Х
Dichlorobromomethane	5.0	X	X	Х
1,1-Dichloroethane	5.0	Х	Х	Х
1,2-Dichloroethane	5.0	Х	Х	X
1,1-Dichloroethylene	10.0	X	Х	Х
1,2-Dichloroethylene	10.0	Х	X	· X
Dichloromethane	10.0	Х	Х	Х
1,2-Dichloropropane	5.0	Х	Х	Х
cis-1,3-Dichloropropene	20.0	Х	Х	Х
trans-1,3-Dichloropropene	s 5.0	Х	Х	Х
GEthylbenzene	10.0	Х	190.	· X
1,1,2,2-Tetrachloroethane	10.0	Х	Х	X
Tetrachloroethylene	5.0	Х	Х	Х
G Toluene	5.0	27.1	31.7	Х
1,1,1-Trichloroethane	5.0	Х	X	Х
1,1,2-Trichloroethane	5.0	Х	Х	X
Trichloroethylene	5.0	Х	Х	Х
Vinyl Chloride	20.0	Х	Х	Х
Trichlorofluoromethane	10.0	х	Х	Х
Dichlorodifluoromethane	20.0	Х	Х	X
m & p-Xylene	10.0	73.8	130.	i X
o-Xylene	10.0	150.	220.	X
Analytical No.		12915	12916	12917

X = Analyzed but not detected

NALYTICAL JREPORT



FOR:

ENVIRONMENTAL AND ANALYTICAL SERVICE

WISCONSIN LAB CERTIFICATION NO. 737053130

Northern	Environmental Technologies
W61 N313	Washington Ave.
P.O. Box	532
Cedarburg	, WI 53012

P.O. #:	Project	No.	BRA	10041
SAMPLE		Clie		
DATE R	EC'D:	1-30)-89	
REPORT	DATE:	2-14	4-89	
APPRO\	/ED BY: 🦾	udu	Lur	andowsk.
	1	7	7	

Re: BRA 10041

VOC Analysis (ng/g or ppb)

	Detectin Limit	<u>DB-4</u>
Benzene	2.0	х
Bromoform	8.0	X
Bromomethane	16.0	X
Carbon Tetrachloride	2.0	X
Chlorobenzene	8.0	X
Chloroethane	8.0	X
2-Chloroethylvinyl Ether	20.0	X
Chloroform	2.0	Х
Chloromethane	8.0	Х
Dibromochloromethane	2.0	Х
l,2-Dichlorobenzene	4.0	Х
1,3-Dichlorobenzene	4.0	Х
1,4-Dichlorobenzene	4.0	Х
Dichlorobromomethane	2.0	Х
1,1-Dichloroethane	2.0	Х
1,2-Dichloroethane	2.0	Х
1,1-Dichloroethylene	4.0	Х
1,2-Dichloroethylene	4.0	Х
Dichloromethane	4.0	Χ.
1,2-Dichloropropane	<u>~</u> 2.0	Х
cis-1,3-Dichloropropene	8.0	Х
trans-1,3-Dichloropropene	2.0	Х
Ethylbenzene	4.0	Х
1,1,2,2-Tetrachloroethane	4.0	X
Tetrachloroethylene	2.0	Х
Toluene	2.0	Х
1,1,1-Trichloroethane	2.0	Х
l,l,2-Trichloroethane	2.0	Х
Trichloroethylene	2.0	X
Vinyl Chloride	8.0	X
Trichlorofluoromethane	4.0	Х
Dichlorodifluoromethane ·	8.0	Х
m-Xylene	4.0	Х
o & p-Xylene (as o-Xylene)	4.0	Х
Analytical No.	A :	12918

X = Analyzed but not detected

NAN AY'D (CALLR DP()R∉I

FOR:



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WISCONSIN LAB CERTIFICATION NO. 737053130

P.O. #:	
SAMPLED BY:	Client
DATE REC'D:	1-30-89
REPORT DATE:	2-13-89
APPROVED BY:	1991
APPROVED BT: _	

Northern Environmental W61 N313 Washington Ave. P.O. Box 532 Cedarburg, WI 53012

Attn: John Dellaport

	Inorg	ganic Ana.	lysis (mg,	/1)	
	Supply Well	MW-1	MW-2	<u>MW-3</u>	Detection Limit
Arsenic	x	x	X	X	0.005
Chromium	X	x	x	x	0.009
Copper	x	0.023	х	x	0.005
Zinc	0.008	0.012	0.004	0.016	0.003
Analytical No.	12914	12915	12916	12917	

e.

X = Analyzed but not detected

NADYFICAL REPORT

P.O. Box 532

FOR:

ENVIRONMENTAL AND ANALYTICAL

WISCONSIN LAB CERTIFICATION NO. 737053130

P.O. #:	
SAMPLED BY:	Client
DATE REC'D:	1-30-89
BEPORT DATE:	2-14-89
APPROVED BY:	111
APPROVED DT.	

ЧC

Cedarburg, WI 53012

Northern Environmental W61 N313 Washington Ave.

Attn: John Dellaport

Re: Ashland Site

Inorganic Analyses (ug/g)*

	DB-4	Detection Limit
Arsenic	100.	22.
Chromium	5.	2.
Copper	46.5	1.
Zinc	165.	0.6
Analytical No.	12918	

* Calculated on the dry weight basis

NA BY NCALIRIDPORT

FOR:



ENVERONMENTAL AND ANALYTICAL SERVICES WISCONSIN LAB CERTIFICATION NO. 737053130

Northern Environmental Technologies	P.O. #: <u>Proj. BRA 10041</u>
P.O. Box 532	SAMPLED BY: <u>Client</u>
Cedarburg, WI 53012	DATE REC'D: <u>1-27-89</u>
Attn: John Dellaport	REPORT DATE: 2-16-89 APPROVED BY:

Ashland Site Assessment

	Detection Limit	<u>MW-1</u>	<u>MW-2</u>	<u>MW-3</u>
BOD ₅ , mg/l	6.	14.	14.	42.
COD, mg/l	10.	330.	224.	1870.
SO ₄ , mg/l	0.3	3.6	5.5	4.6
C1, mg/1	0.1	54.8	64.6	64.1
NPOC, mg/1	0.1	29.0	25.2	27.1
Analytical No.		12831	12832	12833

ę.

X = Analyzed but not detected

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▲ Northern Environmental Technologies Incorporated

APPENDIX A2

FECAL COLIFORM RESULTS ASHLAND TREATMENT PLANT LABORATORY

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FECAL COLIFORM

ASHLAND WATER UTILITY ASHLAND, WISCONSIN

BACTERIOLOGICAL EXAMINATION OF WATER

pH of Presumptive Broth

6.8

pH of Confirmed Broth 6-9

Time & Date Sampled	Sampling Point	Collected By:	Time Inocu- lated	ML. Used	Presu 24 hr	nptive 48 hr	Confirm 24 hr	
1-25-89	mw-1	John Dellaport	1-26-89	10	+			
			9:15A	10	+			
			* .	10	+			
	RACK L	to Rack	C	10	+	-	_	
				10	+		-	•
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ASHLAND WATER UTILITY ASHLAND, WISCONSIN

BACTERIOLOGICAL EXAMINATION OF WATER

pH of Presumptive Broth _

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6.8

pH of Confirmed Broth

6.9

Time & Date Sampled	Sampling Point	Collected By:	Time Inocu- lated	ML. Used		mptive 48 hr	Confirmed 24 hr 48 hr		
1-25-89	MW2	John Dellaport	1-26-89	10	+				
			9:209	10	+				
			7	10	K				
	RACK	C = D		10	K				
				10	+				
			:	1	+				
	MPN=	- 0		1	+				
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ASHLAND WATER UTILITY ASHLAND, WISCONSIN

BACTERIOLOGICAL EXAMINATION OF WATER

pH of Presumptive Broth ____

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FECTFORM

6.8

pH of Confirmed Broth

Time & Date Sampled	Sampling Point	Collected By:	Time Inocu- lated	ML. Used	Presul 24 hr	nptive 48 hr	Confirr 24 hr	ned 48 hr
1-25-85	mw-3	John Dellaport	1-26-89	10	+			
		Della port	9:25A	10	L			
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▲ Northern Environmental Technologies

APPENDIX B

SOIL EXPLORATION BORING LOGS

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Incorporated

APPENDIX B1

BOREHOLE LOGS NORTHERN ENVIRONMENTAL TECHNOLOGIES, INC., JANUARY 1989

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Sheet 1 of 2

NORTHERN ENVIRONMENTAL TECHNOLOGIES SOIL EXPLORATION BOREHOLE LOG ABBREVIATIONS AND SYMBOLS

Column Headings

DEPTH: Depth in feet below natural ground surface.

SAMPLE: Sample identification number.

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- BLOWS: Number of hammer blows required to drive a standard split barrel sampling device six inches (ASTM 1586).
- RECOV: Split barrel sample recovery in inches, maximum recovery is 18 inches (ASTM 1586).
- % GAS: Per-cent combustible gas by volume recorded in the uppermost auger flight immediately before sample collection.
- PID: Photoionization detector soil headspace reading (ppm).
- SYMBOL: Graphic representation of major soil categories and well construction materials as shown below.

Soil Fill

Wood Waste Fill



Fine to Coarse Sand, Beach Deposit



Sandy Silty Clay with Gravel, Lacustrine Deposit



Nonbedded Gravelly Clay with Sand and Silty Sand, Till Deposit

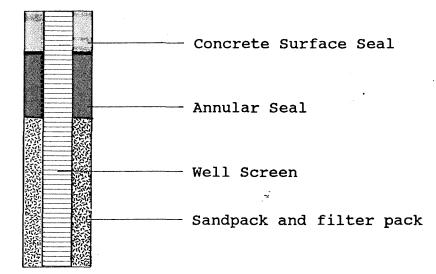


Coarse Graded Sand, Gravel and Clay Till, Outwash Deposit

Sheet 2 of 2

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NORTHERN ENVIRONMENTAL TECHNOLOGIES SOIL EXPLORATION BOREHOLE LOG ABBREVIATIONS AND SYMBOLS



Miscellaneous Abbreviations

OD:	"Outside diameter".
ID:	"Inside diameter" or "identification".
sd:	"Site Datum".
EOB:	"End of boring".
EA:	"Earth auger".
HSA:	"Hollow stem auger".
ppm:	"Parts per million".
10YR 5/3:	Representative Munsell color notation.
CL, GC, etc.:	Unified soil classification group symbol.

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BOREHOLE LOG

Sheet 1 of 2

Project : City of Ashland (BRA10041) Location: Ashland, Wisconsin

Borehole ID: SB1 Logged by : J.C. Dellaport

l											in City	Start	End
_	aeration tanks GROUND ELEV: 606.50 TOTAL DEPTH: 16.0 feet							ks	RIG: 0	Te CME 55	sting	DATE: 01/18 1989	01/18 1989
							feet		BIT(S)		5-inch ID HSA 5-inch OD	TIME: 1430	1515
1		BOREHOLE DIA: 6.25 inch						h	FLUID: None COMPLETED AS: Monitoring Wel			1	
-		D E P	S A M	B L O	R E C	% G	P I D	S Y M			MATERIAL DESCR	ΖΤΡΨΤΟΝ	
(т Н	P L E	W S	o V	A S		B O L		*9	AND COMMEN	1	
1	0.0 -		S1				70		0.0 -	0.0 - 0.5: DARK BROWN SILT LOAM TO roots and rounded pebbl creosote or napthalene	ded pebbles, s	les, slight	
1	1.							17.95V			Fill).	penarene odor;	(01,
1	1.							1.45514	0.5 -	3.5:	HETEROGENEOUS DARK BROWN SAN BROWN CLAY, (5	DY CLAY AND RE	DDISH
	2.			2							wood fragments, moist, soft, strong creosote like odor (Fill, Wood Waste).		
1	3.	0 -	S2	3 5	6		6.0						
-	3.	5 -		5					3.5 -	13.0:	WOOD WASTE, pr wood fibers an		
-	4. 4.							1 1			oily staining surfaces, some clay. At 7.0 f	dark brown si eet, primarily	a
-	5.		S3	1.	7		72				yellow green s fiber mass, ex Larger wood sl	treme creosote	odor.
	5.	5 -		2					-		9.5 and 12.0 f Waste).		
-	6. 6.										-		
-	7.												
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-	8.	5						<u> </u>					

Incorporated

Sheet 2 of 2

BOREHOLE LOG

Project : City of Ashland, (BRA10041) Location: Ashland, Wisconsin

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	LO	CATI		60 f				DRILLER: Twin City Start End	a
	GRO	DUND		aera V: 6			KS		/18
-	ТО	FAL I	DEPT	H: 1	6.0 :	feet		BIT(S): 3.25-inch ID HSA TIME: 1430 15	
-	BOI	REHO!	LE D	IA:	6.25	inc	h	6.25-inch OD FLUID: None COMPLETED AS: MW Monitoring Well	-1
-	D E P T H	S A M P L E	B L O W S	R E C O V	% G A S	P I D	S Y M B O L	MATERIAL DESCRIPTION AND COMMENTS	
-	8.5 - 9.0 -								
-	9.5 -				<u> </u>				
	10.0-								
-	10.5	S5	1	NR					
_	11.0-		1						.
_	11.5							13.0 - 16.0: SANDY CLAY, reddish brown, sof some rounded 1/8 to 1/2-inch	τ,
_	12.0-							arkosic sandstone pebbles and some subrounded 4-inch gabbroi	
	12.5-		1	·			-	pebbles, moist, medium toughne (2.5-5YR 3/4) (CL - CH, Rework	ed
-	13.0-	S6	1	5		8.0	运 二	Glacial Till, Lacustrine, Mill Creek Formation).	er
	13.5		1					NOTE: 1) Wood fragments often became wedged	in
-							-	shoe of split spoon as sampler was driven. This prevented full sampler	
	14.0-							recovery and an exact determination	
-	14.5-		3					of interval changes (i.e. S5). 2) Static Water Level:	
	15.0	S7	4	16		9.5		01/18/89 ≈ 5 feet below grade. 3) Monitoring well (MW-1) installed i	n
-	15.5		7					boring on 01/19/89.	
دمع	16.0-							EOB: 16.0 Feet	
-							-		

Incorporated

Sheet 1 of 2

BOREHOLE LOG

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Project : City of Ashland, (BRA10041)
 Location: Ashland, Wisconsin

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	moi GRO TO	nito OUND FAL I	cing ELEV DEPTH	150 f well V: 60 H: 16	MW- 06.43	-1 3 feet		RIG: BIT(S	Tes CME 55	in City sting 5-inch ID HSA 5-inch OD	DATE: 01 19 TIME: 19 COMPLETI	989 530 ED AS	
	D E P T H	S A M P L E	B L O W S	R E C V	% G A S	P I D	S Y M B O L		.7	MATERIAL DESCH AND COMMEN		ing We	211
1 1	0.0 - 0.5 - 1.0 -	S1				8.1		0.0 -	4.5:	GRAVELLY SILT, (5YR 2.5/1), d moist, soft, s rounded pebble (OL, Fill, May Wastewater Tre	iry to sli some wood es, root i be filte	ightly fragn fragme er cal	/ ments, ents ke from
4	1.5 - 2.0 -		2									,	•
1	2.5 - 3.0 - 3.5 -	S2	1 1	NR		8.9							
(4.0 - 4.5 -		2					4.5 -	10.8:	WOOD WASTE, mo	hips and	fibro	ous
1	5.0 - 5.5 -	S3	6 7	3		12				fragments, str odor, larger w (Fill).	rong creos vood slabs	s at 1	ase
	6.0 - 6.5 -											-	
~	7.0 -	S4	2 5	3		25							
	8.0 - 8.5 -		8										

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Sheet 2 of 2

BOREHOLE LOG

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- Project : City of Ashland, (BRA10041) Location: Ashland, Wisconsin

<u>____</u>

-				150 f well			of	DRILLER: Twin City Start End Testing					
				V: 60				RIG: CME 55 DATE: 01/18 01/18 1989 1989					
	TOT	TAL I	DEPTI	H: 10	5.0 :	feet		BIT(S): 3.25-inch ID HSA TIME: 1530 1615 6.25-inch OD					
-	BOI	REHOI	LE D	IA: (5.25	incl	ı	FLUID: None COMPLETED AS: MW-2 Monitoring Well					
-	D E	S A	B L	R E	98	P	S Y						
	P T	M P	O W	C O	G A	D	M B	MATERIAL DESCRIPTION AND COMMENTS					
-	т Н	Р L E	s S	V	S		O L						
، هم	.5 -						2 -						
9	.0 -						<u> </u>						
- g	. 5 -						[2][1]	10.8 - 11.0: SAND, somewhat poorly graded, fine to medium grained, brown					
			7				<u> </u>	(5YR 3/4), moist, soft, subrounded, trace fine gravel					
-	.0.0-	S5	7	7		21		(SP, Beach Deposit, Miller Creek Formation).					
			11				<u> </u>						
-	1.0-							11.0 - 13.5: SILTY SAND, brown (5YR 3/4), fine to medium grained, moist, soft,					
1	.1.5-							some rounded pebbles up to 4-inch, clayey sand layers,					
	2.0-		4					fining with depth (SP - SM, Lacustrine, Miller Creek					
נ ו	.2.5	S6	8	12		8.6		Formation).					
1	.3.0-		11										
_ 1	.3.5-							13.5 - EOB: SANDY SILT, brown (5YR 3/4), with some fine subrounded pebbles up					
1	.4.0-							to 注-inch, firm, dry to slightly moist (ML, Glacial Till,					
- 1	.4.5-							Lacustrine, Miller Creek					
1	.5.0-		5					Formation).					
	5.5	S7 7 12 9.0											
_ ¹	.6.0-							EOB: 16.0 Feet					
								NOTE: 1) Monitoring Well MW-2 installed in boring on 01/20/89.					
-													

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Sheet 1 of 2

BOREHOLE LOG

Project : City of Ashland, (BRA10041)
 Location: Ashland, Wisconsin

	GR(TO	CATIO OUND TAL I REHOI	ELEV DEPTH	arte: 7: 6: H: 10	sian 10.3 [°] 6.0 :	wel] 7 feet	L	DRILLER RIG: CM BIT(S): FLUID: 1	Tes E 55 3.25 6.25	n City sting 5-inch ID HSA 5-inch OD	TIME: COMPLI	Start 01/19 1989 1308 ETED AS oring We	
((D E P T H	S A M P L E	B L O W S	R E C O V	% G A S	P I D	S Y M B O L		2	MATERIAL DESCE AND COMMEN			
0 - 1).0 -).5 - 0 -	Sl				8.0		0.0 - 2	.0:	GRAVELLY CLAYE to black (2.59 heterogeneous, gravel, moist, odor (OL, Fill	R 3/0) some : soft,	, organ: fine ang	ic, gular
2 - 2 3 - 3 4	· · 0 · · 5 · 0 · 5 · 5	S2	2 1 2	8		5.8		2.0 - 5	.5:	SILTY SAND AND (5YR 3/1), wel with rounded f 1-inch angular moist, soft. F heterogeneous rounded pebble with 1 inch la moist wood chi SM, Fill).	l grade ine pel glass Sottom clay w es, sof	ed silty bbles, a y slag o 12 incho ith somo t, moist degrado	y sand chunk, es is e t, ed
- 5 5 6 7 7 8	.0 .5 .5 .5 .5 .5 .5 .0 .5	S3 54	1 2 2 2 2 2 8	8		7.2 9.6		5.5 - 1		WOOD WASTE, up dry to moist, chunks and fra 10.5, wood is greenish color odor, oily she boulder or cob	dark s agments more ma , slig en on v	tained w . From a oist, s ht petro wood su	wood 8.5 to trong pleum rfaces,

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Sheet 2 of 2

BOREHOLE LOG

Project : City of Ashland, (BRA10041) Location: Ashland, Wisconsin

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<u>-</u>	LO	CATI			eet s			DRILLER: Twin City Start End	1
_	GR	OUND			sian 10.3'		L	Testing RIG: CME 55 DATE: 01/19 01/ 1989 198	
	то	TAL I	DEPTI	H: 10	6.0 :	feet		BIT(S): 3.25-inch ID HSA TIME: 1308 150 6.25-inch OD	
-	BOI	REHO	LE D	IA:	6.25	incl	n	FLUID: None COMPLETED AS: MW- Monitoring Well	-3
[D E P T H	S A M P L	B L O W S	R E C O V	% G A S	P I D	S Y M B O	MATERIAL DESCRIPTION AND COMMENTS	
		E					L		
_	8.5 - 9.0 -						1 - 1 - 1		
	9.5 -								
-	10.0-	S5	*					* Blow Counts S5: 100/12 inches	
	10.5							10.5 Feet: Boulder or cobble pushed to side	≥.
_	11.0-							10.5 - EOB: SAND, thinly bedded layers of subrounded fine sand, medium sar	
	11.5-							and clayey coarse to very coarse sand. Each layer is poorly grade	
	12.0-		11					brown (5YR 3/4), moist to saturated, soft (SW, Beach	
_	12.5	S6	21	14		9.0		Deposit, Miller Creek Formation)	•
	13.0		25						
_	13.5						• • • • •		
	14.0-							· .	
	14.5		12						
_	15.0-	S7	22	18		8.8			
	15.5		29						
-	16.0-							EOB: 16.0 Feet	- 1 7
-								NOTE: 1) Sand caved into hole and against we screen (10.5 - 15.0 feet) during monitoring well MW-3 installation.	311

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BOREHOLE LOG

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Project : City of Ashland, (BRA10041)
 Location: Ashland, Wisconsin

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- [CATIC	sed	iment	atio			DRILLER: Tw Te: RIG: CME 55	sting	Start End DATE: 01/20 01/20
-		TAL I				feet			5-inch ID HSA - 14.5': None	1989 1989 TIME: 1200 1535
-	BO	REHOI	LE DI	IA: (5.25	incl	ı	14.5 - 61.0 Slurry		COMPLETED AS: Soil Boring
-	D E P T H	S A M P L E	B L O₩ S	R E C V	% GAS	P I D	S Y M B O L	2	MATERIAL DESCH AND COMMEN	
- 0	.0 -							0.0 - 1.5:	to black, mois	(SILT, Dark brown st, soft, low
	•5 -	S1							plasticity, so angular red bu heterogeneous (5YR 2.5/1) (N	ome 1/2 to 1 inch rick fragments, 1L, Fill).
_	.5 -							1.5 - 3.5:	GRAVELLY SAND, medium sand wi grains, a few	, Brown fine to ith some very coarse rounded red arkosic
-	.5 -	S2	6 3	12					diameter. Son	oles up to 1 inch ne clayey sand layers 3.5/3) (SP - SC,
	.5		4					3.5 - 11.0:	SANDY CLAY, re	eddish brown and
-	.0 - .5 -								toughness, het	no odor, medium cerogeneous, trace s at 7 ft. and 1/4" (CL, Fill).
_ 5	.0 -	6.2	3	9						
5	.5 -	S3	з 5	9		_				i
	.0 -									
_	.5									
	.0 -		1							
^	.0 -	S4	2	4		-				
- 8			2							

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BOREHOLE LOG

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Project : City of Ashland, (BRA10041)
 Location: Ashland, Wisconsin

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	GRO TO:	DUND	sed: ELEV DEPTI	iment V: 60 H: 61	of 1 tatio 06.8 1.0 1 5.25	on ta feet	anks	DRILLER: Twin City Testing RIG: CME 55 BIT(S): 3.25-inch ID HSA FLUID: 0.0 - 14.5': None 14.5 - 61.0: Bentonite Slurry	Start End DATE: 01/20 1989 1989 TIME: 1200 COMPLETED AS: Soil Boring
1 (D E P T H	S A M P L E	B L O W S	R E C O V	% G A S	P I D	S Y M B O L	MATERIAL DESC AND COMMEN	RIPTION
1	8.5 - 9.0 - 9.5 -								4
-	10.0- 10.5-	S5	3	7					
(11.0- 11.5-		3					brown to blad	ch layer of dark ck (5YR 2.5/1),
-	12.0- 12.5-	S6						shale chips a fragments, ma	ace flat ½-inch and blocky coal oist, soft. Enclosed h layer of reddish
-	13.0-							brown, soft,	moist, fat clay inch wood fragment
-	13.5- 14.0-							14.5 - 35.5: GRAVELLY SIL	
	14.5 1 15.0							¹ / ₂ -inch round elongate san	moist, no odor, ed dolomite and red dstone pebbles, trace
-	15.5-	S7	5 10					toughness and 3/4) (CL, Gla	omogeneous, medium d plasticity, (5YR acial Till, Lacu-
-	16.0- 16.5-							strine, Mille NOTE: S6, 11.5 - 13.5 fee sample, bottom of to black pebbly clay an	ube, dark brown to
-	17.0-							slight petroleum od	

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BOREHOLE LOG

- Project : City of Ashland, (BRA10041) Location: Ashland, Wisconsin

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-	LO	CATIC	ON: sed	West imen	of : tatio	fina on t	l anks	DRILLER: Twin City Testing	Start	End
	GR	DUND		V: 6				RIG: CME 55	DATE: 01/20 1989	01/20 1989
	TO	FAL I	DEPT	H: 6	1.0	feet		BIT(S): 3.25-inch ID HSA FLUID: 0.0 - 14.5': None	TIME: 1200	1535
-	BOI	REHO	LE D	IA:	6.25	inc	h	14.5 - 61.0: Bentonite Slurry	COMPLETED AS: Boring	Soil
	D	s	В	R E	%	P I	S Y			
	E P	A M	L O	c	G	D	M	MATERIAL DESC AND COMMEN		
_	T H	P L	W S	o V	A S		B O	AND COMMEN	N15	
		E					L			
_ 1	.7.0-									
1	17.5					· · · · · · · · · · · · · · · · · · ·				4
- 1	.8.0-									
נ	L8.5-									
– 1	L9.0-									
_1	L9.5-									
2	20.0-		1		ļ					
-2	20.5-	S8	2	18						
	21.0-		3							
_	21.5									
_	2.0-									
	2.5									1
-2	23.0-									
2	23.5	•								
7 2	24.0-									
_2	24.5		 							
2	25.0-		1	10						
-2	25.5-	S9	3	18						
		۱	L	L	L	L	1			

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BOREHOLE LOG

- Project : City of Ashland, (BRA10041) Location: Ashland, Wisconsin Docation: Ashland, Wisconsin

-	GRO TO	DUND TAL I	sed: ELE DEPTI	West iment V: 60 H: 61 IA: 0	tatio 06.8 1.0 :	on ta feet	anks	DRILLER: Twin City Testing RIG: CME 55 BIT(S): 3.25-inch ID HSA FLUID: 0.0 - 14.5': None 14.5 - 61.0: Bentonite Slurry COMPLETED AS: Boring	End 01/20 1989 1535 Soil
1 1	D E P T H	S A M P L E	B L O W S	R E C O V	% G A S	P I D	S Y M B O L	MATERIAL DESCRIPTION AND COMMENTS	
$ \begin{array}{c} 2 \\ - 2 \\ 2 \\ - 2 \\ - 2 \\ - 2 \\ - 2 \\ - 2 \\ - 3 $	25.5- 26.0- 26.5- 27.0- 27.5- 28.0- 29.0- 29.5- 30.0- 31.0- 31.5- 31.0- 31.5- 31.0- 31.5- 31.0- 31.5- 31.0- 31.5- 31.0- 31.5- 31.0- 31.5- 31.0- 31.5- 31.0- 31.5- 31.0- 31.5- 31.0- 31.5- 31.0- 31.5- 31.0- 31.5- 31.0- 31.5- 31.0- 31.5- 31.0- 31.5- 31.0- 31.5- 31.5- 31.0- 31.5- 31.5- 31.0- 31.5- 31.0- 31.5- 31.0- 31.5- 31.5- 31.0- 31.5- 31	S9	3	NR					

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BOREHOLE LOG

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Project : City of Ashland, (BRA10041)
 Location: Ashland, Wisconsin

	GR TO	OUND TAL	sed ELE DEPT	imen V: 6 H: 6		on t feet	anks	DRILLER: Twin City Testing RIG: CME 55 BIT(S): 3.25-inch ID HSA FLUID: 0.0 - 14.5': None 14.5 - 61.0: Bentonite Slurry	Start End DATE: 01/20 01/20 1989 1989 TIME: 1200 1535 COMPLETED AS: Soil Boring Soil
-	D E P T H	S A M P L E	B L O W S	R E C O V	% G A S	P I D	S Y M B O L	MATERIAL DESCI AND COMMEN	
	34.0 34.5 35.5 35.5 36.0 36.5 37.0 37.5 38.0 37.5 38.0 39.0 39.5 40.0 1.0 1.0 1.5	S11	2 2 5 	18				above, moist, rounded 1½-ir pebble (2.5 -	, more sand than , red degraded hch sandstone - 10YR 3/6) and ch green sandstone Slacial Till,
4	2.0								

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BOREHOLE LOG

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-Project : City of Ashland, (BRA10041) Location: Ashland, Wisconsin

-				imen	tatio	fina on ta		DRILLER: Twin City Testing RIG: CME 55	Start DATE: 01/20	End 01/20
-						feet		BIT(S): 3.25-inch ID HSA	1989 TIME: 1200	1989 1535
-	BOI	REHO	LE DI	[A: 0	5.25	incl	n	FLUID: 0.0 - 14.5': None 14.5 - 61.0: Bentonite Slurry	COMPLETED AS: Boring	Soil
-	D E P T H	S A M P L E	B L O W S	R E C V	% G A S	P I D	S Y M B O L	MATERIAL DESCI AND COMMEN		
	2.5									
	3.0- 3.5-									ı
	4.0-									
	4.5 ⁻		15							
-4	5.5-	S13	21 24	4						
	6.0-									
_	7.0-									
-	7.5-									
	18.5-						6			i
_	9.0-									
	50.0-		10							
	50.5-		16 24	9		7.4				
-5	51.0-									

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BOREHOLE LOG

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Project : City of Ashland, (BRA10041)
 Location: Ashland, Wisconsin

	-						T		
-	LOC	CATIC				fina on ta		DRILLER: Twin City Start Testing	End
_	GRO	DUND					GTILD	RIG: CME 55 DATE: 01/20	01/20 1989
	TO	CAL I	DEPTI	H: 6	1.0 :	feet			1535
_	вог	REHOI	LE D	IA:	6.25	incl	h	14.5 - 61.0: Bentonite COMPLETED AS:	Soil
				[1	T	1	Slurry Boring	
-	D E	S A	B L	R E	olo	P I	S Y		:
	P T	M P	O W	С 0	G A	D	M B	MATERIAL DESCRIPTION AND COMMENTS	
-	н	L E	S	v	S		O L		
_	51.0-								
	51.5-								
-	52.0-								
	52.5-								
-	53.0-								
	53.5-								
	54.0-							BLOW COUNTS S15: 100/15"	
_	54.5							54.5 - EOB: SAND WITH CLAY AND GRAVEL,	
	55.0-							well graded, moist, soft, ang to subangular, 3/4-inch gabbr	ular oic
-	55.5	S15	*	10		8.9	0.000	and sandstone pebbles, 2.5YR (SW-SC, Glacial Outwash, Copp	3/4
								Falls Formation).	
-	56.0-				-		00000		
	56.5						000	i	
^	57.0-								
_	57.5-								
	58.0-					<u> </u>			
~	58.5-						0.00		
	59.0-								
_	59.5-				}				

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BOREHOLE LOG

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Project : City of Ashland, (BRA10041)
 Location: Ashland, Wisconsin

-	LOO	CATIC	DN: N sed:	Vest	of : tatio	fina on ta	l anks		1	Twin City Testing		Start	End
		DUND	ELEV	V: 60	06.8			RIG:				01/20 1989	01/20 1989
		TAL I						FLUID): 0.0	.25-inch ID HSA) - 14.5': None	TIME:		1535
-	BOI	REHOI	LE DI	IA: (6.25	inc	h	14.5 Slurr		.0: Bentonite	COMPL Borin	ETED AS: g	: S011
-	D E P T H	S A M P L E	B L O W S	R E C O V	% G A S	P I D	S Y M B O L			MATERIAL DESC			
		<u>Б</u>											
	59.5-						0000 00000 00000	BLOW	COLINI	rs s16: 100/10"			
	60.0- 60.5-	S16	*	6		7		DTOM	COON.	10 510. 100/10			;
	61.0-						0.0.0	EOB:	61.0	feet			
_	-							NOTE :	1)	At EOB (61.0 fee flowing artesia: minute flow.	et), bo n, ¹ z-g	ring was allons p	s. per
_	-						-		2)	Photoionization faulty. PID va this boring.	detect lues ar	or opera e suspec	ation ct for
	-								3)	Boring abandone	d and s	ealed of	1
-										1/23/89. Set b 20 feet. Tremi slurry to surfa	e grout	e prug a ed cemen	nt
_	· _												
	-						-						:
_	-												i
	-												
_													
	-												
-	-												
	-												
—	-						$\frac{1}{2}$						

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BOREHOLE LOG

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Project : City of Ashland, (BRA10041)
 Location: Ashland, Wisconsin

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-	GRO TO BOJ	OUND FAL I REHOI 0 - 3	a ELEV DEPTH LE DJ L3.5	aerat 7: 60 1: 61 [A: ft:	cion 05.4 1.0	south tank feet 5 inc 375 i	cs ch	RIG: CME 55 BIT(S): 0.0-13.5': 3.25 ID HSA	End 01/25 1989 1415 Soil
-	D E P T H	S A P L E	B L O ₩ S	R E C V	% G A S	P I D	S Y M B O L	MATERIAL DESCRIPTION AND COMMENTS	
- 1 - 1 - 2 - 2	0.0 - 0.5 - 0 - 2.5 - 2.5 -	S1 	2 3	11		2.2		0.0 - 3.5: CLAYEY SILT, reddish brown (5YR 3/3) slightly moist, sof some trace sand at top, finer depth, trace fine gravel, heterogeneous (CL, Fill).	t, with
4 - 4 - 5 5 - 6 - 7 - 7 8		S3	4 1 2 3 1 1 2	8		0		3.5 - 12.1: CLAY, reddish brown, 5YR 4/4 very moist plastic clay with fine gravel, heterogeneous, d brown organic clay pockets, (2.5/1) (CL-CH, Fill).	trace ark

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Sheet 2 of 5

BOREHOLE LOG

Project : City of Ashland, (BRA10041) Location: Ashland, Wisconsin

	GRO TOT BOH 0.0	DUND FAL I REHOI	ELE DEPTI LE DI 13.5	ft:	tion 05.4 1.0 : 6.2!	tan feet 5 in	ks	DRILLER:Twin City Testing Start End RIG: CME 55 DATE: 01/17 01/25 BIT(S): DATE: 01/17 01/25 0.0-13.5': 3.25 ID HSA 1989 1989 13.5-61.0': 2.875 Tricone TIME: 1100 1415 FLUID: 0.0 - 13.5': None COMPLETED AS: Soil Soil Slurry Boring Date: 01/17 01/25
-	D E P T H	S A P L E	B L O W S	R E C O V	% G A S	P I D	S Y M B O L	MATERIAL DESCRIPTION AND COMMENTS
-	3.5 - 9.0 - 9.5 -							S5: 8.5 - 10.5: THIN WALLED TUBE SAMPLE FOR GEOTECHNICAL ANALYSIS
- - 1	10.0- 10.5- 11.0- 11.5-			12				
- 1	12.0- 12.5-	S6	1	7		11		12.1 - 12.4: WOOD WASTE, greenish - black moist stained wood chips and fragments, strong creosote-like odor, (Fill).
- 1	L3.0- L3.5- L4.0-	50	2			<u>+</u> +		12.4 - 12.6: SAND, brown stained medium, poorly graded san, creosote-like odor, saturated (SP, near shore, Reworked Glacial Till, Beach Deposit, Miller Creek Formation).
- 1	14.5- 15.0-	S7	2	17		10		12.6 - 16.0: CLAY, reddish brown, (2.5YR ⁴ /4) moist, soft, plastic, with trace fine gravel and sand (CL - CH, Glacial Till, Lacustrine, Miller Creek Formation).
_ 1	15.5- 16.0- 16.5-		3					NOTE: 16.0 - 61.0 feet logged by Twin City Testing Corporation 16.0 - 24.5: FAT CLAY WITH SAND, red, moist, medium to stiff, (CH, Possible
_	L7.0-							Fill).

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BOREHOLE LOG

Project : City of Ashland, (BRA10041) Location: Ashland, Wisconsin

- GROUN TOTAL BOREH 0.0 -	ION: 15 aen D ELEV: DEPTH: OLE DIA: 13.5 ft - 61.0 f	ration 1 605.4 61.0 fe : : : 6.25	tanks eet inch	DRILLER:Twin City Testing Start End RIG: CME 55 DATE: 01/17 01/2 BIT(S): DATE: 01/17 01/2 0.0-13.5': 3.25 ID HSA 1989 1989 13.5-61.0': 2.875 Tricone TIME: 1100 1415 FLUID: 0.0 - 13.5': None COMPLETED AS: Soil Soil Slurry Boring Date: 01/17 01/2	25
D S E A P M T P H L E	F	G G A	P S I Y D M B O L	MATERIAL DESCRIPTION AND COMMENTS	
$ \begin{array}{c} -17.0\\ 18.0\\ -19.0\\ 20.0\\ -20.0\\ 58\\ 21.0\\ -20.0\\ $				 24.5 - 26.0: SILTY SAND, with a little gravel fine-grained, reddish-brown, moist, very dense (SM, Beach Deposit). 26.0 - 34.0: FAT CLAY WITH SAND, red, moist, rather stiff, trace of gravel (CH, Lacustrine). 	

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BOREHOLE LOG

_	Project :	City of	Ashland,	(BRA10041)
	Location:	Ashland	, Wisconsi	n

-				aerat	tion			DRILLER:Twin City Testing RIG: CME 55	Start	End
_	GRO	DUND	ELEV	7: 60	05.4			BIT(S): DATE: DATE: DATE:	01/17 1989	01/25 1989
			DEPTI		1.0 1	feet		13.5-61.0': 2.875 Tricone TIME: FLUID: 0.0 - 13.5': None		1415
_	0.0	b – 1	LE DI 13.5	ft:	6.25	5 in	ch	13.5 - 61.0: Bentonite COMPLI	ETED AS:	Soil
	13	5 -	61.0) ft	: 2.8	375	inch	Slurry Boring	3	
_	D	S	N	R	8	P	S Y			
	E P	A M	v	E C	G	L D	м	MATERIAL DESCRIPTION		
	T H	P L	AL	0 V	A S		B	AND COMMENTS		
		E					L			
	34.0-							34.0 - 43.0: SILTY SAND, with a	little g	ravel,
-	35.0-							reddish-brown, fine- moist, very dense (S		
		S11	37					Till).		1
_	36.0-									
	37.0-									
	38.0-									
	39.0-									
	40.0~	S12	78							
. <u></u>	41.0-									-
	42.0-									
-	43.0-						<u>n:00</u>	43.0 - 56.0: POORLY GRADED SAND		
	44.0-			ļ			0.00	with a little grave brown, fine-grained		.sh-
-							0.00	bearing, very dense	(SP - S	SM,
	45.0-	S13	79				0.00	Glacial Outwash).		i
-	46.0-						0.00			
	47.0-						0.00			
-	48.0-									
							0.2.0 0.00 0.00 0.00	·		
_	49.0-		1				0.00 0.00 0.00			
	50.0-	S14	70				00000			
-	51.0-	014								
					A		H . CANA			

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Sheet 5 of 5

BOREHOLE LOG

Project : City of Ashland, (BRA10041) Location: Ashland, Wisconsin

D S N R % P S I Y Y Y M Y C G D M - T P A O A B AND COMMENTS - S1.0 - - - - - - 52.0 - - - - - - - 52.0 - - - - - - - - 53.0 - - - - - - - 54.0 - - - - - - - - 55.0 - <t< th=""><th></th><th>LOCATIO GROUND FOTAL I BOREHOI 0.0 - 2 13.5 -</th><th>ELEV DEPTI LE DI L3.5</th><th>aerat V: 60 H: 61 IA: ft:</th><th>tion 05.4 1.0 1 6.25</th><th>tan feet 5 in</th><th>ks ch</th><th>DRILLER:Twin City Testing Start End RIG: CME 55 DATE: 01/17 01/25 BIT(S): DATE: 01/17 01/25 0.0-13.5': 3.25 ID HSA 1989 1989 13.5-61.0': 2.875 Tricone TIME: 1100 1415 FLUID: 0.0 - 13.5': None COMPLETED AS: Soil Soil Slurry Boring Date: 01/17 01/25</th></t<>		LOCATIO GROUND FOTAL I BOREHOI 0.0 - 2 13.5 -	ELEV DEPTI LE DI L3.5	aerat V: 60 H: 61 IA: ft:	tion 05.4 1.0 1 6.25	tan feet 5 in	ks ch	DRILLER:Twin City Testing Start End RIG: CME 55 DATE: 01/17 01/25 BIT(S): DATE: 01/17 01/25 0.0-13.5': 3.25 ID HSA 1989 1989 13.5-61.0': 2.875 Tricone TIME: 1100 1415 FLUID: 0.0 - 13.5': None COMPLETED AS: Soil Soil Slurry Boring Date: 01/17 01/25
52.0 53.0 53.0 54.0 55.0 515 80 56.0 56.0 57.0 56.0 58.0 56.0 59.0 56.0 60.0 516 62 61.0 510 510 56.0 58.0 56.0 59.0 511 60.0 516 62 61.0 511 510 511 511 511 512 512 513 512 513 512 52.0 512 53.0 512 53.0 512 53.0 512 53.0 512 53.0 512 53.0 512 53.0 512 54.0 513 55.0 513 55.0 513 514 514 515 514 515 514 515 514 514 514 <td></td> <td>E A P M F P H L</td> <td>V A</td> <td>E C O</td> <td>G A</td> <td>I</td> <td>Y M B O</td> <td>AND COMMENTS</td>		E A P M F P H L	V A	E C O	G A	I	Y M B O	AND COMMENTS
grouted with a cement bentonite	52. 53. 54. 55. 55. 57. 58. 59. 60.	.0 .0 .0 .0 .0 .0 .0 .0 .0 .0 .0 .0 .0						<pre>reddish-brown, fine-grained, moist, very dense (SM, Glacial Till).</pre> EOB - 61.0 feet NOTE: 1) Field compressive strength on soil sample S3; 0.25 - 0.3 SQ. FT. 2) Portion of soil sample S6 submitted for laboratory analysis of base/ neutral and acid extractable compounds. 3) Static Water Level: 8 feet below grade 4 hours after drilling to 16.0 feet. 4) Boring was temporarily cased to 13.5 feet (4½" OD steel casing with (1) threaded coupling). Annulus was

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BOREHOLE LOG

- Project : City of Ashland, (BRA10041) Location: Ashland, Wisconsin

	Va GR TO BO 0.	CUUM DUND FAL I REHOI	DN: 2 filt ELEV DEPTH LE DI 16.0 61.0	ter & 7: 60 H: 61 [A: ft:	build 06.2 1.0 : 6.2!	ding feet 5 ind	ch	DRILLER:Twin City Testing Start End RIG: CME 55 DATE: 01/19 01/2 BIT(S): DATE: 01/19 01/2 0.0-14.5': 3.25 ID HSA 1989 1989 14.5'-61.0':2.875 Tricone TIME: 1045 1130 FLUID: 0.0 - 16.0': None COMPLETED AS: Soil Soil Slurry Boring Date: 01/19 01/2
1	D E P T H	S A M P L E	B L O W S	R E C V	% G A S	P I D	S Y M B O L	MATERIAL DESCRIPTION AND COMMENTS
-1	.5	S1				7		0.0 - 2.0: GRAVELLY SILTY CLAY, heterogeneous medium brown (5YR 3/3) and dark brown (5YR 3/1), moist, soft, some root matter and trace fine coal chips (CL, Topsoil and Fill).
2 2	.0 -	S2	3 9 4	12		7.4		2.0 - 4.7: GRAVELLY CLAYEY SILT AND PEAT, day brown (5YR 2.5/1) soft organic si and reddish brown (2.5YR 4/6) mois clayey silt. Black rooty peat and wood chips at base (6") (ML, Fill)
-4 _5	.0 .5 .0	S3	2 2 4	5		11		4.7 -10.7: WOOD WASTE, moist greenish-black stained wood chips and fibrous fragments. Mild creosote - like odor. Saturated and water bearing (Fill).
6 7 7	.0 .5 .5 .0	S4	4 2 4	5		12		
- 8	.5							

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Sheet 2 of 8

BOREHOLE LOG

Project : City of Ashland, (BRA10041)
 Location: Ashland, Wisconsin

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9. 9. 9. 10 10 11 11 11 12	Vacu GROU TOTA BORE 0.0 16.0 D E P T H	L DEPT HOLE D - 16.0 - 61.0 S B A L M O P W L S E	ter b V: 60 H: 61 IA: ft:	eet SE uilding 6.2 .0 feet 6.25 in 2.875 % P I G D A S	ch	DRILLER:Twin City Testing RIG: CME 55 BIT(S): 0.0-14.5': 3.25 ID HSA 14.5'-61.0':2.875 Tricone FLUID: 0.0 - 16.0': None 16.0 - 61.0: Bentonite Slurry MATERIAL DESCRIPTION AND COMMENTS
9. - 9. - 10 - 11 - 11 - 12	E P P H .5 .0 .5	A L M O P W L S E	E C O	I G D A	Y M B O	AND COMMENTS
9. - 9. - 10 - 11 - 11 - 12	.0				26.0	
- 10 - 11 11 - 12	0.0-	1 2		· · · · · · · · · · · · · · · · · · ·		
	0.5 1.0 1.5 2.0	2 5 1 9 7	5	14		10.7 - 29.5: SANDY SILT WITH SAND, reddish brown fine to medium saturated sand layer (3") overlying firm slightly moist reddish brown (2.5YR 3/4) sandy silt. Some rounded weathered mafic pebbles 1/8 - 1/2 inch, trace clay in matrix (SC - ML, Lacustrine,
- 13 - 13 14	3.0 3.5 4.0	6 8		9.2		Miller Creek Formation). NOTE: Northern Environmental Hydrogeologist observed drilling to 16.0 feet and logged soil samples, collected by driller, to 61.0 feet.
- 15 - 15	4.5 5.0 5.5 6.0	20 7 21 26		8.2		

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Sheet 3 of 8

BOREHOLE LOG

- Project : City of Ashland, (BRA10041) Location: Ashland, Wisconsin

					······					
		ON:					DRILLER:Twin City Testing	st	art	End
		fil			ding		RIG: CME 55		(1.0	
GR	ROUND	ELE	V: 6	06.2			BIT(S):	DATE: 01	/19 89	01/2
					с .		0.0-14.5': 3.25 ID HSA	19 TIME: 10		1989 1130
		DEPT		1.0 :	reet		14.5'-61.0':2.875 Tricone FLUID: 0.0 - 16.0': None	IIME: IO	45	1130
		LE D 16.0		6 21	5 in	ch	16.0 - 61.0: Bentonite	COMPLETE	D AS:	: Soil
16	. n –	61.0	0 ft	: 2.8	875	inch	Slurry	Boring	(
		1	, <u> </u>	r			1			
D	s	N	R	8	P	s				
Е	A		E		I	Y				
Ρ	М	v	С	G	D	M	MATERIAL DESCR			
Т	P	A	O V	A		B O	AND COMMEN	115		
Н	LE	L	V	S		L				
7.0)	<u> </u>								
7.5	;		<u> </u>							;
8.0)			t						
	.									
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9.0	·		ļ		ļ					•
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	S8	50								•
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3.5			ļ	<u> </u>						
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4.5	;+	<u> </u>								
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5.0										
	S9	48								
5.5	·									

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BOREHOLE LOG

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	City of Ashland, (BRA10041) Ashland, Wisconsin	
	-	

- Va(GR(TO) BOI 0.0	CATIO CUUM DUND FAL D REHOL D - 1 .0 -	filt ELE\ EPTH E DJ 6.0	ter 1 7: 60 H: 61 [A: ft:	ouilo 06.2 1.0 1 6.25	ling feet 5 in	ch	DRILLER:Twin City Testing Start End RIG: CME 55 DATE: 01/19 01/25 BIT(S): DATE: 01/19 01/25 0.0-14.5': 3.25 ID HSA 1989 1989 14.5'-61.0':2.875 Tricone TIME: 1045 1130 FLUID: 0.0 - 16.0': None COMPLETED AS: Soil Boring
- D E P T H	S A M P L E	N V A L	R E C V	% G A S	P I D	S Y M B O L	MATERIAL DESCRIPTION AND COMMENTS
-25.5 26.0 26.5 27.0 27.5 28.0 28.5 -29.0 29.5 -30.5 30.5 31.0 -31.5 32.0 -32.5 33.0 -33.5 -34.0	S10 2	22					 29.5 - 31.0: SAND AND SILTY CLAY, reddish brown (2.5YR 3/4) thinly bedded fine poorly graded sand layers (1/2") alternating with reddish brown silty clay (3"), trace fine gravel, moist, firm (SP - SC, reworked Glacial Till, Miller Creek Formation). 31.0 - 39.5: COARSE SAND, reddish brown (2.5YR 3/4) well graded fine to very coarse sand, some silt and fine gravel (SW Glacial Outwash, Copper Falls Formation).

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BOREHOLE LOG

_	Project :	City of Ashland, (BRA10041)	
	Location:	Ashland, Wisconsin	

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	TOT BOF	CUUM DUND FAL I REHOI	filt ELE DEPTH LE DI 16.0	ter] V: 60 H: 61 [A: ft:	feet ouild 06.2 1.0 f 6.25 : 2.8	ling feet 5 in	ch	DRILLER:Twin City Testing Start End RIG: CME 55 DATE: 01/19 01, BIT(S): DATE: 01/19 01, 0.0-14.5': 3.25 ID HSA 1989 198 14.5'-61.0':2.875 Tricone TIME: 1045 113 FLUID: 0.0 - 16.0': None COMPLETED AS: Sor Sor Slurry Boring Date: Start Date: 01/19	/25 39 30
[[D E P T H	S A M P L E	N V A L	R E C V	% G A S	P I D	S Y M B O L	MATERIAL DESCRIPTION AND COMMENTS	
•	34.0- 34.5-	-							
-	35.0-	S11	19				000000000000000000000000000000000000000		
-	35.5- 36.0-						0.000 0.000 0.000 0.000 0.000		
-	36.5-								
	37.0- 37.5-								
(38.0- 38.5-								
	39.0-						0.000		
	39.5 40.0-							39.5 - 54.5: SANDY SILT AND SILTY SAND, firm reddish brown (2.5YR 3/4) sandy silt with fine angular gravel.	Y
	40.5	S12	80					Grades into a soft silty sand a 44.5 feet (ML, Glacial Till and Outwash, Copper Falls Formation	1 E
	41.0-						0.000		-
-	41.5 42.0						00,00		
	42.5-						0.000		

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BOREHOLE LOG

Project : City of Ashland, (BRA10041)
 Location: Ashland, Wisconsin

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_	Va GR TO	CUUM DUND FAL 1	ON: 2 filt ELEV DEPTH LE D	ter 1 V: 6 H: 6	buil0 06.2	ding		DRILLER:Twin City Testing RIG: CME 55 BIT(S): 0.0-14.5': 3.25 ID HSA 14.5'-61.0':2.875 Tricone FLUID: 0.0 - 16.0': None	DATE: TIME:		End 01/25 1989 1130
-	0.0) – :	16.0	ft:	6.2 2.8	5 in 875 :	ch inch	16.0 - 61.0: Bentonite Slurry	COMPLI Boring	ETED AS:	Soil
-	D E P T H	S A P L E	N V A L	R E C V	% G A S	P I D	S Y M B O L	MATERIAL DESCE AND COMMEN			
-	42.5- 43.0-										4
-	43.5- 44.0-						000000				
-	44.5-										•
-	45.0- 45.5-	S13	101				00000000000000000000000000000000000000				
-	46.0- 46.5-						0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.0				
•	47.0-						00000000000000000000000000000000000000				
_	48.0-						000 000 000 000 000				i
_	48.5 49.0						0.000000000000000000000000000000000000			•	
	49.5- 50.0-						0,000,00 0,000,00 0,000,00				
-	50.5	S14	59				0.00 0.00 0.00 0.00 0.00 0.00 0.00 0.0				
	51.0-				<u> </u>		0. 2.				

BOREHOLE LOG - Project : City of Ashland, (BRA10041) Borehole ID: 88-3 Logged by : J.C. Dellaport Location: Ashland, Wisconsin Start End DRILLER: Twin City Testing LOCATION: 110 feet SE of vacuum filter building RIG: CME 55 DATE: 01/19 01/25 BIT(S): GROUND ELEV: 606.2 1989 0.0-14.5': 3.25 ID HSA 1989 14.5'-61.0':2.875 Tricone TIME: 1045 1130 TOTAL DEPTH: 61.0 feet FLUID: 0.0 - 16.0': None BOREHOLE DIA: COMPLETED AS: Soil 16.0 - 61.0: Bentonite 0.0 - 16.0 ft: 6.25 inch Slurry Boring 16.0 - 61.0 ft: 2.875 inch R % Ρ S D S Ν Е Ι Y Ε Α С D М MATERIAL DESCRIPTION Ρ v G Μ AND COMMENTS В Ο A \mathbf{P} Т Α v S 0 Η L L \mathbf{L} Ε 51.0-51.5-52.0-52.5-53.0-53.5 54.0-54.5 - 59.0: GRAVEL WITH SILT AND SAND, well-- 54.5graded, reddish brown (2.5YR 3/4), with rounded pebbles up to 1-inch, 55.0saturated, soft, with silt and S15 60 sand (GW - GM, Glacial Outwash, 55.5 Copper Falls Formation). 56.0-56.5-- 57.0-57.5 58.0-59.0 - EOB: SANDY SILT WITH GRAVEL, firm, 58.5 reddish brown (2.5YR 3/4), moist with fine angular gravel 59.0-(ML, Glacial Till, Copper Falls

Formation).

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_ 59.5[.]

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BOREHOLE LOG

Project : City of Ashland, (BRA10041)
 Location: Ashland, Wisconsin

	TO: BOI	CUUM CUND FAL I REHOI	fil: ELE DEPTI LE DI 16.0	ft:	build 06.2 1.0 : 6.2!	ding feet 5 in		RIG: BIT(S 0.0-1 14.5' FLUID	CME): 4.5' -61. : 0.	5! .0 .0	in City Testing 5 3.25 ID HSA ':2.875 Tricone - 16.0': None D: Bentonite Slurry	DATE: TIME:	ETED AS:	End 01/25 1989 1130 : Soil
-	D E P T H	S A M P L E	N V A L	R E C O V	% G A S	P I D	S Y M B O L				MATERIAL DESC AND COMME			
-	59.5-						00							;
-	60.5-	.	72							_	- ·			
-	61.0-							EOB: NOTE:			feet Boring was temp	orarily	cased t	- 0
_								NOIE:	I)	1	with threaded co was grouted with slurry which cu	DD stee oupling n cemen	l casing). Annu t/bentor) 11us nite
-	-								2)		Boring was flow 51.0 feet (1/25,		esian at	-
-									3))	l/-/89: Abando	ned Bor	ing.	
_							-							
	-													i
-	-													
-	_													
	_													
-	_													
_	_													

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BOREHOLE LOG

Project : City of Ashland, (BRA10041) Location: Ashland, Wisconsin

											
-	LO	CATI	ON: 3						n City Testing	Start	End
					tion	tan	ks	RIG: CME 55		DATE: 01/24	01/24
_	GR	OUND	ELEV	/: 60	16.5			BIT(S): 0.0-17.5': 3	.25 ID HSA	1989	1989
-	то	TAL	DEPTI	H: 43	1.0 1	feet		17.5-41.0':	2.875 Tricone	TIME: 0820	1130
	BO	REHO	LE D	EA:					17.5': None		0.41
1			17.5					17.5 - 41.0: Slurry	Bentonite	COMPLETED AS: Boring	SOIL
		.5 -	41.0		. 2.0	5/5	inch	Sturry		boring	
	D	s	В	R	8	Р	s				
ł	E	A	L	E		I	Y		MATERIAL DESCH	ντρπτω	
	P T	M P	O W	C O	G A	D	M B		AND COMMEN		
_	H	L	S	v	s		ō	ۇب. ئەر			
		Е					L				
	0.0							0.0 - 1.0:	GRAVELLY silt.	dark brown 5Y	'R
_	0.0	S1				15			2.5/1 with sar	nd and clay.	
	0.5								Saturated, sof		
									organic (ML, H	nct sewage odc	er,
ſ	1.0								NOTE: Describe		
	1.5	ļ							cuttings.		
-										CLAY, heterog R 2.5/2) and r	
	2.0		5						brown (7.5YR 4	(6) angular ma	fic
	2.5	ļ							pebbles up to	1/2 inch, orga	nic,
-		S2	2	1		3				inch slab frag	ments
	3.0								moist, soft (C	CL, Fill).	
_	3.5	ļ	3								
	5.5										
	4.0										
l	4.5						1994 (SV)	4 5 - 10.8:	WOOD WASTE, mo	oist wood chunk	s.
	4.5		1 1				$\mathcal{O}(\mathcal{U};\mathcal{H})$	4.5 10.0.	slabs and fibr	rous fragments,	tan
	5.0						1014			slightly moist	
_		S3	2	5		-			slight creosot	ce - like odor, eet. Water bea	wood
	5.5		1				13.7%		zone, voids (H	fill).	;
-	6.0	 	<u>↓</u> <u> </u>				STAT.				
							1.50				
	6.5		1				1231				
-	7.0	ļ	1				£92.9				
			26				1840分				
	7.5					25					
-	8.0	S4	4	5		35	11/2				
	0.0		5				1.6.73				
_	8.5										
		L	L	L	I	I	pr-c//c				

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Sheet 2 of 4

BOREHOLE LOG

- Project : City of Ashland, (BRA10041) Location: Ashland, Wisconsin

	GRO TO BOI 0.0	DUND	ELE ELE DEPTI LE DI 17.5	aerat V: 60 H: 41 IA: ft:	tion 06.5 1.0 : 6.2!	5 ind	s	DRILLER:Twin City Testing Start End RIG: CME 55 DATE: 01/24 01/24 BIT(S): DATE: 01/24 01/24 0.0-17.5': 3.25 ID HSA 1989 1989 17.5-41.0': 2.875 Tricone TIME: 0820 1130 FLUID: 0.0 - 17.5': None COMPLETED AS: Soil Soil Slurry Boring Date: 01/24 01/24
-	D E P T H	S A P L E	B L O W S	R E C O V	% G A S	P I D	S Y M B O L	MATERIAL DESCRIPTION AND COMMENTS
9 - 9 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1 - 1		S5	4 8 8 4 6 10 7	6		10		<pre>10.8 - 16.0: SILTY SAND AND SANDY SILT, medium bedded poorly graded reddish brown (2.5YR 3/4) silty fine sand soft, moist, alternating with poorly graded reddish brown (2.5YR 3/4) moist sandy silt, soft to firm, trace rounded sand- stone pebbles up to ½-inch (SP - SM, ML, Beach Deposit, Miller Creek Formation).</pre>
- 1	5.5- 6.0-	S7	11 19	14		5.5		16.0 - 25.0: SANDY LEAN CLAY, reddish brown, moist very still, trace of gravel (CL, Lacustrine, Miller Creek
	6.5- 7.0-		· · ·					Formation). NOTE: 16.0 - 41.0: Logged by Twin City Testing Corporation

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BOREHOLE LOG

Project : City of Ashland, (BRA10041)
 Location: Ashland, Wisconsin

										r		
-	GRO TO: BOI 0.0	DUND FAL I REHOI	ELEV DEPTI LE DI 17.5	aerat 7: 60 H: 41 [A: ft:	tion 06.5 1.0 1 6.25	tan feet 5 in	ks	RIG: CME 55 BIT(S): 0.0-17.5': 3 17.5-41.0':	2.875 Tricone - 17.5': None	DATE: TIME:	ETED AS:	End 01/24 1989 1130 Soil
-	D E P T H	S A M P L	N V A L	R E C V	% G A S	P I D	S Y M B O		MATERIAL DESCI AND COMMEN			
		Е					L					
-	17.0- 18.0-											1
-	19.0-		-									
_	20.0-	S8	38									
	21.0 ⁻ 22.0 ⁻											·
	23.0-											
_	24.0-											
	25.0-						20.00.0	25.0 - 41.0:	POORLY GRADE	D SAND,	with gr	ravel,
~~ ~	26.0-	S9	17				8. 9. 9. 1 3000		reddish-brown grained, wate very dense. 1	er bear	ing, der	nse to
-	27.0-						0000		from 35 feet encountered	Artesi (Glacia	an flow 1 Outwas	
	28.0-						0.0.0		Copper Falls	Format	ion).	i
-	29.0-						0.000					
	30.0-	S10	45				5.000 č					
-	31.0-						\$000 000 000 000 000 000					
-	32.0-						0000					
	33.0-											
-	34.0-						0.00					

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BOREHOLE LOG

Project : City of Ashland, (BRA10041)
 Location: Ashland, Wisconsin

-	GRO TO: BOI 0.0	DUND FAL I REHOI	ELE ELE DEPTI LE D 17.5	ft:	tion 06.5 1.0 : 6.2!	tan feet 5 in	ks	RIG: CME BIT(S): 0.0-17.5 17.5-41.0 FLUID: 0.	Twin City Testing 55 ': 3.25 ID HSA)': 2.875 Tricone .0 - 17.5': None L.0: Bentonite	Start DATE: 01/24 1989 TIME: 0820 COMPLETED AS Boring	End 01/24 1989 1130 : Soil
•	D E P T H	S A M P L	N V A L	R E C O V	% G A S	P I D	S Y M B O		MATERIAL DESCH AND COMMEN		
		Е					L				
	34.0-										
_	-	S11	53				0.000				i
	36.0-										
_	37.0-						0.00.0 0.00.0 0.00.0				
	38.0-						0.00 0.00 0.00				
	39.0-						0000				
	40.0-	S12	34				0.0.0 .0.0 .0.0 .0.0 .0.0 .0				
-	41.0-						7.0.0.C	EOB 41.0	Feet		
-								NOTE: 1)	Boring was logged Northern Environ Boring was comple geotechnical purp	mental Geologis eted at 41.0 f	st. eet for
_									Testing Corporat:	ion, 01/24/89.	CICY
_								2)	Boring was tempor feet (4 1/2" OD s threaded coupling	steel casing w	ith (2)
									grouted with a construct which cure	ement/bentonit	e
-								3)	01/24/89: Abando	oned boring.	
_					<u> </u>						
-											

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BOREHOLE LOG

Project : City of Ashland, (BRA10041)
 Location: Ashland, Wisconsin

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$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	_ 1	L.5 ·		4					5YR 4/4 (CL, Fill). 1.5 - 2.9: SAND, fine to medium, poorly graded reddish sand, moist, ro	ound-
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	3	3 . 0 -	S2		7		4.9		(SP, Fill). 2.9 - 6.0: PEAT AND WOOD WASTE, black, ro moist peat overlying greenish- black stained wood fragments,	ooty -
$\begin{array}{c c c c c c c c c c c c c c c c c c c $	4	1.5		1					odor, up to 2 inch wood chunks wood is light, fibrous and de-	5,
6.5 2 stained wood chips and fragments, strong creosote like odor, larger wood slabs with depth (Fill). 7.0 4 7.5 3			S3		6		14			
- S4 3 7 7 7.0 4 - - 7.5 3 -				2					stained wood chips and fragmen	nts,
			S4	3	7		7		wood slabs with depth (Fill).	LYEL
				4						
	Name.	3.0 -		3						
			S5	1	3		35			

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BOREHOLE LOG

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Project : City of Ashland, (BRA10041)
 Location: Ashland, Wisconsin

-	TOT BOF 0.0	TAL I REHOI	ELE ELE DEPTI LE DI 18.0	artes V: 60 H: 41	sian)7.1 L.0 1 6.2!	feet 5 ind	l ch	RIG: CME 55 BIT(S): 0.0-13.5': 3.25 ID HSA DATE: 01/18 03 1989 19	nd 1/24 989 645 oil
-	D E P T H	S A P L E	B L O W S	R E C V	% G A S	P I D	S Y M B O L	MATERIAL DESCRIPTION AND COMMENTS	
- 8	.5	S 5	3						
9	.0 -		5						
- 9	.5 -	S6	5	4		28			
- 10	0.0-		11						
10	0.5-		3				· · · · · · · ·	10.6 - 12.5: SAND, well graded, fine, media and coarse, reddish brown (5Y)	R
_ 11	1.0-	S7	10	10		2.2	0. 0.	4/3) saturated, subrounded, was some subrounded quartz pebbles	s up
	1.5		11				0.00	to ¹ /-inch, trace silt (SW, Bea Deposit, Miller Creek Formatic	
	2.0-		5						
-	2.5	S8	16	10	<u> </u>	11		12.5 - 26.0: SILT, reddish brown, (5YR 3/3) more dry, firm some very coars	se
	3.0-		19					sand and fine angular pebbles, some thin fine - medium sand beds. Grades into a clayey s:	
_	3.5-	:	11					with pebbles. Sandy silt at I (ML, Lacustrine, Miller Creek	base
	4.0	S9	15	14		13		Formation).	
	4.5 5.0-		25					,	
	5.5								
	6.0-								
_	6.5-								:
	7.0-								

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Sheet 3 of 5

BOREHOLE LOG

- Project : City of Ashland, (BRA10041) Location: Ashland, Wisconsin

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$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	GRO TO BOI 0.0	DUND FAL I REHOI D - 2	ELE ELE DEPTI LE DI 18.0	ft:	sian 07.1 1.0 : 6.2!	wel: feet 5 in	1	DRILLER:Twin City Testing RIG: CME 55 BIT(S): 0.0-13.5': 3.25 ID HSA 13.5'-41.0':2.875 Tricone FLUID: 0.0 - 18.0': None 18.0 - 41.0: Bentonite Slurry COMPLETED AS: Boring	End 01/24 1989 1645 Soil
17.5 18.0 18.5 19.0 19.5 15 20.0 510 20 14 20.5 37 21.0 37 21.5 22.0 22.5 23.0 23.5 23.0	E P T	A M P L	L O W	E C O	G A	I	Y M B O	AND COMMENTS	
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	17.5- 18.0- 18.5- 19.0- 19.5- 20.5- 21.0- 21.5- 22.0- 22.5- 23.0- 23.5- 25.5-	S10	20 37					than 4.5 tons per square foot. 23.0 - 24.0: SCHIST BOULDER	eater

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Sheet 4 of 5

BOREHOLE LOG

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Project : City of Ashland, (BRA10041)
 Location: Ashland, Wisconsin

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	GRO TOT BOH 0.0	DUND FAL I REHOI	ELE ELE DEPTI LE DI L8.0	arte: V: 60 H: 41 IA: ft:	1.0 ±	wel: feet 5 ind	1	DRILLER:Twin City Testing Start End RIG: CME 55 DATE: 01/18 01/24 BIT(S): DATE: 01/18 01/24 0.0-13.5': 3.25 ID HSA 1989 1989 13.5'-41.0':2.875 Tricone TIME: 0825 1645 FLUID: 0.0 - 18.0': None COMPLETED AS: Soil Soil 18.0 - 41.0: Bentonite Boring Date: 01/18 01/24
	D E P T H	S A P L E	B L O W S	R E C V	% G A S	P I D	S Y M B O L	MATERIAL DESCRIPTION AND COMMENTS
_ 2	25.5-							25.2 - 26.0: SCHIST BOULDER
2 -3 -3	26.0- 27.0- 27.5- 28.0- 28.5- 29.0- 29.5- 29.5-	 	11	17		4.2		26.0 - EOB: SAND, brown (5YR 3/4) fine, poorly graded, rounded sand, saturated, with rounded quartz and angular gabbroic pebbles up to 1/2 inch. 1-inch rounded red sandstone pebble. Sequence coarsens with depth to a well graded sand. Lower 10 feet con- tains more sandstone and gabbroic pebbles up to 1-inch (Glacial Outwash, Copper Falls Formation).
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Sheet 5 of 5

BOREHOLE LOG

Project : City of Ashland, (BRA10041) Location: Ashland, Wisconsin

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- GR TO BO - 0.	CATION OUND TAL DI REHOLI 0 - 13 .0 - 2	a ELEV EPTH E DI 8.0	artes 7: 6(1: 4: [A: ft:	sian)7.1 L.0 f 6.25	wel: feet 5 inc	l ch	DRILLER:Twin City Testing Start End RIG: CME 55 DATE: 01/18 01/24 D.0-13.5': 3.25 ID HSA 1989 1989 13.5'-41.0':2.875 Tricone TIME: 0825 1645 FLUID: 0.0 - 18.0': None COMPLETED AS: Soil Soil Slurry Boring Darte: 01/18 01/24
- D E P T H	S A M P L E	N V A L	R E C O V	% G A S	P I D	S Y M B O L	MATERIAL DESCRIPTION AND COMMENTS
- 34.0 34.5 35.0 35.5 36.0 - 36.5 37.0 - 37.5 38.0 38.5 - 39.0 39.5 - 40.0 40.5 41.0	S13	*					 * Blow Counts S13: 100/15" NOTE: 1) A soil sample of auger cuttings (0.0 - 10.5 feet) was submitted for laboratory analysis of base/neutral and acid extractable compounds. 2) Split spoon sampled continuously: 4.5 - 15.0 feet. 3) Static water level: ≈5 feet below grade, 01/23/89. 4) 01/23/89: Boring was temporarily cased to 18.0 feet (4.5 inch OD steel casing with threaded coupling). Annulus was grouted with a cement/ bentonite slurry which cured for 24 hours. 5) Boulders encountered at 23.0 and 25.2 feet. Penetrated with a 2.25 inch carbide tipped drag bit. 6) EOB: 41.0 feet; boring was flowing artesian, ≈ 1.8 gallon per minute (gpm) flow rate. 7) 01/24/89: Boring abandoned and sealed. Bentonite plug set at 23.0 feet and cement/bentonite/mud grout tremied to surface. EOB: 41.0 Feet

▲ Northern Environmental Technologies Incorporated

APPENDIX B2

BOREHOLE LOGS TWIN CITY TESTING CORPORATION, JANUARY 1989

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twin city testing

corporation

formerly operated as Lakehead Testing Laboratory

226 NORTH CENTRAL AVENUE P.O. BOX 7166 DULUTH, MN 55807 PHONE 218/628-2295

February 6, 1989

John C Dellaport, Hydrogeologist Northern Environmental Technologies, Inc P O Box 532 Cedarburg, WI 53012

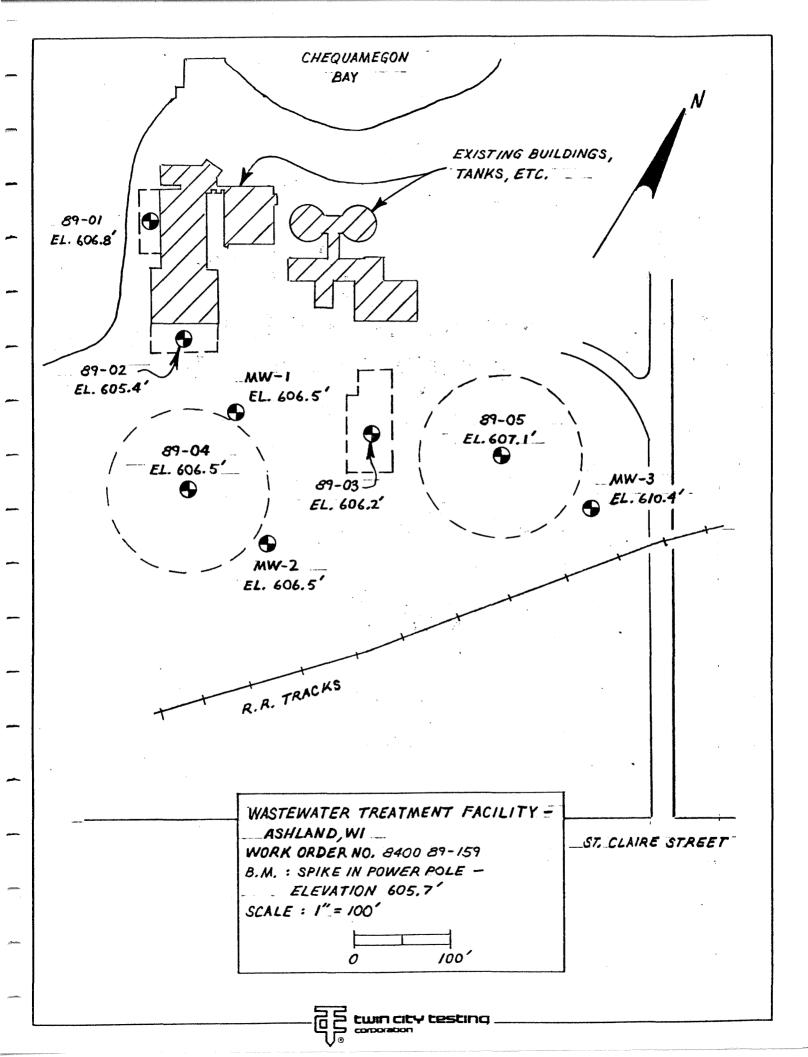
Mr Dellaport,

Enclosed please find photocopies of report logs, driller's boring logs and site plan for the Wastewater Treatment Facility in Ashland, WI, per your request. Please feel free to contact us should you desire additional information.

Sincerely,

Robert L. Kaspsyk

Robert W Kaspszak Earth Scientist



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PROJECT W DEPTH IN FEET SI ORGA ORGA SILT BROW A.S FAT SOF UO SAN 12.6 FAT STI SAN	DESCRIPTION OF MATERIAL DESCRIPTION OF MATERIAL DESCRIPTION OF MATERIAL SURFACE ELEVATION <u>99.4</u> 184'S SAND, BLACK, FING GRAINED, MOIST, W/RGAT, GANICS. (SC) 14'SAND, BLOWN, FINC GRAINED, MOIST, W/RGAT, GANICS. (SC) 14'SAND, BLOWN, FINC GRAINED, MOIST, W/RGAT, JUNT, AND FATCLAY W/SAND, REDDISH - BLOWN, INST, MEDIUM. TRACE OF ORGANICS. (SM) (CN) 15 CLAY W/SAND, RED, MOIST, MEDIUM TO NFT. TRACE OF ORGANICS. DOD CHIPS W/OILY CREDSOTE SMELL, 12'.1 TO 12.4'. ND, BROWN, FING TO MEDIUM GRAINED, MOIST, V/OILY CREDSOTE SMELL, 12.6'. (CH) 15 CLAY W/SAND, RED, MOIST, MEDIUM TO 16 FF. MADE NO.6: TRACE OF BRIGHT ORANGE SURSTANCE; CREOSOTE SMELL, TRACES OF GRAVEL, SILT. LENSES OF WATERBEARING	FILL POJSIBLE	N 7 5 3 4 7	wL	san NO. I Z 3 4 5	APLE TYPE FA 5B 5B 5B 5B 5B 5B 5B 5B	diameter and			EST
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	D 8400 89-159 VERTIC CT WASTEWATER TREATMENT PLANT - ASHL					BORIN	IG NO.	_04	-03	
DEPTH	DESCRIPTION OF MATERIAL			l	SA	MPLE	L	BORA	TORY T	ES
IN FEET	SURFACE ELEVATION	GEOLOGIC ORIGIN	N	WL	NO.	TYPE	w	D		Ť
	0-0.5" DARK BROWN ORGANIC TOPSOIL.				1	FA				\uparrow
-	SANDY SILTY CLAY, BROWN TO BLACK, MOIST,		-		11	r A			1	
4	RATHER STIFF TO MODIUM . W/ PEAT,		ŀ							
	BLACK. CINDERS.		- 11		2	SB				
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	WOOD CHIPS AND SLABS, CREOSOTE SMELL,								l	
-	TRACES OF SAND. WATERBEARING,		-							
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10 -	POORLY GRADED SAND, BROWN, FINE GRAINED,	BEACH	10		5	SB				
-	WATERBARING MENUM DENSE	DEPosit								
12	(42)		F							
	CLAYEY SAND W/GRAVEL, REDDISH- BROWN, FINE GRAINED, MOIST, DENSE		- 27		6	SB			19	
14 -	FINE GRAINES, MOIST, BENSE. (SC)			ļ						
	SANDY LEAN CLAY, REDDISH - BROWN, MOIST,				ł					
-	VERY STIFF TO STIFF TRACE OF GRAVEL.		48		7	SB			;	
-	A FEW COBBLEZ.				`	00				
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IN FEET		CE ELEVATIO	N			0	RIGIN	N	WL	NO.	TYPE	w	D	<u>L.L.</u> P.L.	0
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	SILTY	sand, w/	A LITTLE	GRAVEL	FINE].						
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	FINE GRAINED, MOIST TO WET, LOOSE,		l					
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7-3	WOOD CHIPS AND SLARS,		- 3					
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			['	[[[NSR			
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-			16	5	58			
11 -	SILTY SAND, REDDISH - BROWN, FINE TO		Ţ					1
	MENIUM GRAINED, LIATERREARING, NENSE,	BEACH	F	·				
	TRACE OF ORGANICS. SLIGHT CREOSOTE (SM)	DEPOSIT	- 16		28			
14 ·	Shele-		+ .					
-	SANDY CLAY, REDDISH - BROWN, MOIST,		Ļ					
	VERY STIFF TA STIFF . TRACE OF		30	7	58			
	GRAVEL							
-		LACUSTRINE	Γ					
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-1	POORLY GRADED SAND, EN/GRAVEL, REDDISH-		_ 71 _	۹	58			
-	BROWN, FINE TO COARSE GRAINED WATER-		[•		
-	BEARING, DEASE TO VERY DEASE		F					
-	LAYERS OF SILTY SAND FROM 35		F					;
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	LOG OF TI	EST BORING	3				**************************************		*****
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AND DESCRIPTION OF THE OWNER OF T	CT WASTEWATOR TREATMENT PLANT - ASHL	AND, WI	1	r i.		1	t		
DEPTH IN FEET		GEOLOGIC ORIGIN	N		AMPLE	{}	D	TORY T	
1	SAND, GRAVEL, CLAY, DARK BROWN.		-	1	FA				
-	FINE GRADED SAND W/ SILT, REDDISH-BROWN, FINE GRAINED, MOIST, LOOSE. (SP-SM)	-		2	SB				
2.9 -	PEAT, BLACK, MOIST, W/SAND, ROOTS. (PT)		- 7						
4.5-	WOOD, ROTTEN WOOD, ORGANICS.	FILL	- 3	3	5B				
-	CREOSOTE SMELL.								
-			[/	4	FA				
-			4	2	FA				
10.5		BEACH	- 16	6	FA				
12 -	SILTY SAND, REDDISH - BROWN, FINE GRAINED, WATERBEARING, DENSE. (SM)	DEPOSIT	21	ר	SB				
	SANDY LEAN CLAY, REDDISH -BROWN,		- 35	8	SB				MF
-	MONST, VERY DENSE. TRACE OF GRAVEL. A FEW COBBLES.		L 41	٩	28				
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-	BOULDER, 23 - 24'. CLAYEY GRAVEL, REDDISH - BROWN, WET, VERY	GLACIAL TILL	- 10%2		28				
26.1-	Dense, 24.5-25.2. Boulder, 25.2-21.1. (GC)		- /.2 -						
-0.1	SILTY SAND, REDDICH - BROWN, FINE TO MEDIUM GRAINED, WATELBEARING,		-			ľ			
-	VERY DENSE. TRACE OF GRAVEL.		ŀ					;	
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-	ARTESIAN FLOW ENCOUNTERED AT 40'		ŀ						
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JOB NC	CT WASTE WATER TREATMENT PLANT - ASHLAND, WE														
DEPTH	NO. <u>8400 89-159</u> UERTICAL SCALE <u>1" = 4'</u> BORING NO. <u>89-05</u> UERTICAL SCALE <u>1" = 4'</u> BORING NO. <u>89-05</u> UERTICAL SCALE <u>1" = 4'</u> BORING NO. <u>89-05</u> LABORATORY TESTS SAMPLE LABORATORY TESTS SURFACE ELEVATION <u>SURFACE ELEVATION</u> SILTY SAND (GNT.) GLACIAL OUTWASH 66 14 58														
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APPENDIX B3

BOREHOLE LOGS LAKEHEAD TESTING LABORATORIES, INC., OCTOBER 1970

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LAKEHEAD TESTING LABORATORY, INC.

Novarner 19, 1973



Dresley and Manaen 14 Maat Jackson Rivd. Claugo, Illinois 60004

Rei Swil Investigation Proposed Dewage Treatment Plant Additions & Proposed Elevated Water Storege Tank Ashland, Wiscomfin Ti'66059

Gen lemin!

We have conducted a soil investigation for the proposed Sewage Treatment Plent similing and a soil investigation and foundation analysis for the proposed Blovated Water Storage Tank. We are transmitting two coules of our report. Additional pepies are being and as noted below:

Under superside cover we are sending representative samples of the soil to your erchitects for their personal inspiciou. About 50% of the remaining soil samples will be held at this office for two months and will then be discarded unless we are national to hold them for a longer period of time.

As part of our professional services, we have on our staff formation and noils engineers and angumering geologists available for correct thom. Is the argue of the project develops, they will be available to you to discuss the exercise problems as they arise or aid you in your divisations.

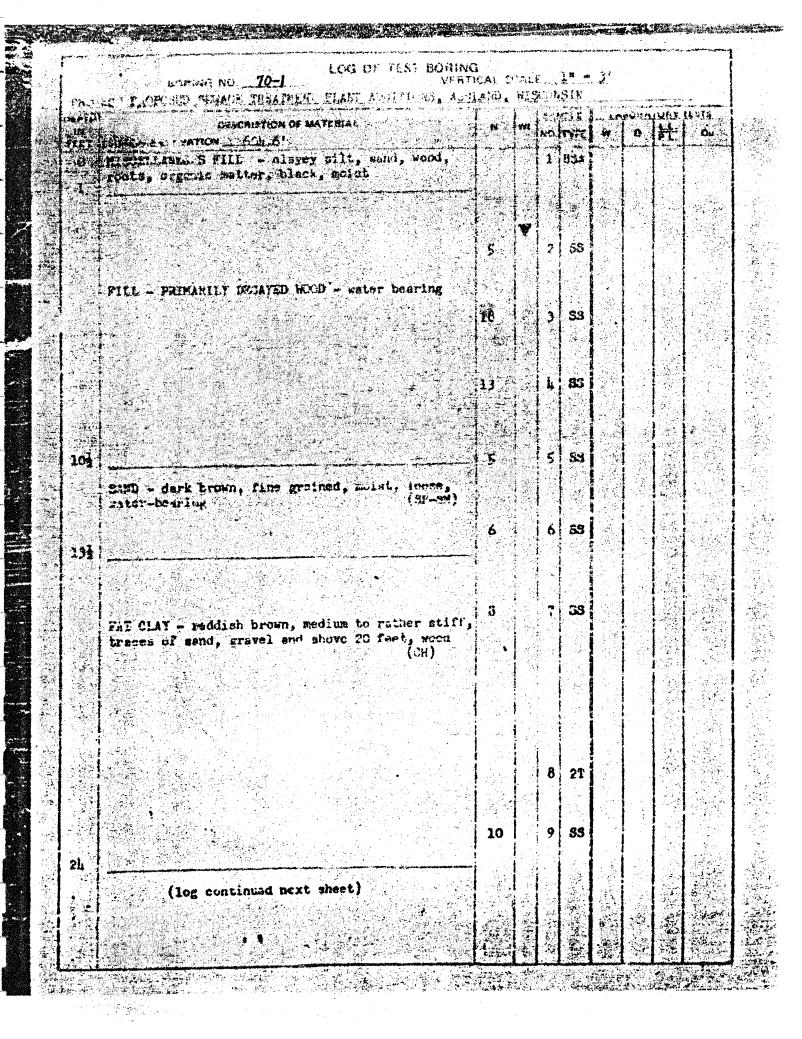
Very truly yours

LARENEAD TESTING LABORATORY.

Lee H. Lbert, F. E.

GC: Ashland Water Utility Attention: Mr. Snow

LHB/1kt



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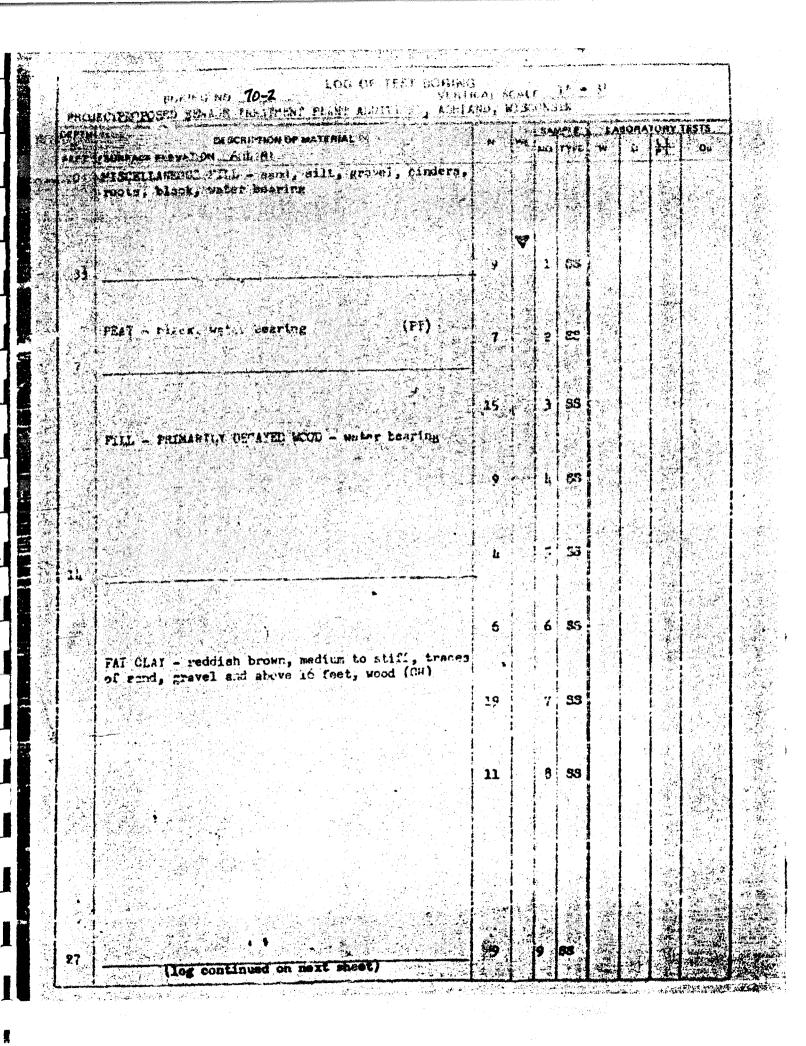
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START 10-6-70 WATER LEVEL MEAGUNEMISTS CON PLETE -10-6 LAKCHEAD METINO 32 HSA to 34 <u>_</u>___ TESTING LABOHATORY, INC. CATH CALING 45 13 S. S. S. S. S. S. 8.c.R. 3. 3' D.15. 12.14 34 8.8* ACR woc. 2.21 to cave in at CHEW CHIEFJ Report AC. 206 8 1. 5r A62.014



27 Senior GLA - brewer mail in work white is fan image val and, a souch of gravel 	HARTH DESCRIPTION	EP MATERIAL					<u>e</u> []	
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APPENDIX F

WATER LEVEL DATA

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WATER LEVEL DATA

Project : City of Ashland (BRA10041)
 Location : Ashland, Wisconsin
 Personnel: J. Dellaport

Well Number : MW-1 Well Location : Riser Elevation : 609.42 Ground Elevation: 606.50

	DATE 1989	TIME	MEASURING DEVICE	DEPTH (ft below top	WATER 1	LEVEL	COMMENTS
-	1909		DEVICE	of riser)	Depth (ft below grade)	Elevation (ft msl)	
-	1/20	1130	OWLP	7.42	4.50	602.00	predevelop
_	1/21	1530	OWLP	7.38	4.46	602.04	predevelop
	1/22	1219	OWLP	7.32	4.40	602.10	predevelop
-	1/23	1055	OWLP	7.36	4.44	602.06	after dev.
•	1/25	1230	OWLP	7.28	4.36	602.14	after dev.
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NOTE: MEASURING DEVICE = OWLP: Olympic Water Level Probe Tape: Steel or Fiberglass Measuring Tape

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WATER LEVEL DATA

 Project : City of Ashland (BRA10041)
 Location : Ashland, Wisconsin Personnel: J. Dellaport

Well Number : MW-2 Well Location Well Location : Riser Elevation : 609.32 Ground Elevation: 606.43

				***		00100000
DATE 1989	TIME	MEASURING DEVICE	DEPTH (ft below top	WATER 1	LEVEL Elevation	COMMENTS
			of riser)	Depth (ft below grade)	(ft msl)	
1/20	1046	OWLP	7.30	4.41	602.02	predevelo
1/21	1530	OWLP	7.25	4.36	602.07	predevelo
1/22	1035	OWLP	7.20	4.31	602.12	predevelo
1/23	1055	OWLP	7.26	4.37	602.06	after dev
1/25	1347	OWLP	7.19	4.30	602.13	after dev
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NOTE: MEASURING DEVICE = OWLP: Olympic Water Level Probe Tape: Steel or Fiberglass Measuring Tape

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WATER LEVEL DATA

Project : City of Ashland (BRA10041)
 Location : Ashland, Wisconsin
 Personnel: J. Dellaport

Well Number : MW-3 Well Location : Riser Elevation : 613.26 Ground Elevation: 610.37

	DATE 1989	TIME	MEASURING DEVICE	DEPTH (ft below top	WATER 2	LEVEL	COMMENTS
-	1989		DEVICE	of riser)	Depth (ft below grade)	Elevation (ft msl)	
	1/20	1613	OWLP	11.24	8.35	602.02	predevelop
-	1/21	1530	OWLP	11.26	8.37	602.00	predevelop
	1/22	0852	OWLP	11.20	8.31	602.06	predevelop
_	1/23	1055	OWLP	11.32	8.43	601.94	after dev.
	1/25	0950	OWLP	11.12	8.23	602.14	after dev.
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NOTE: MEASURING DEVICE = OWLP: Olympic Water Level Probe Tape: Steel or Fiberglass Measuring Tape ▲ Northern Environmental Technologies / n c o r p o r a t e d

APPENDIX C

MONITORING WELL CONSTRUCTION SUMMARIES

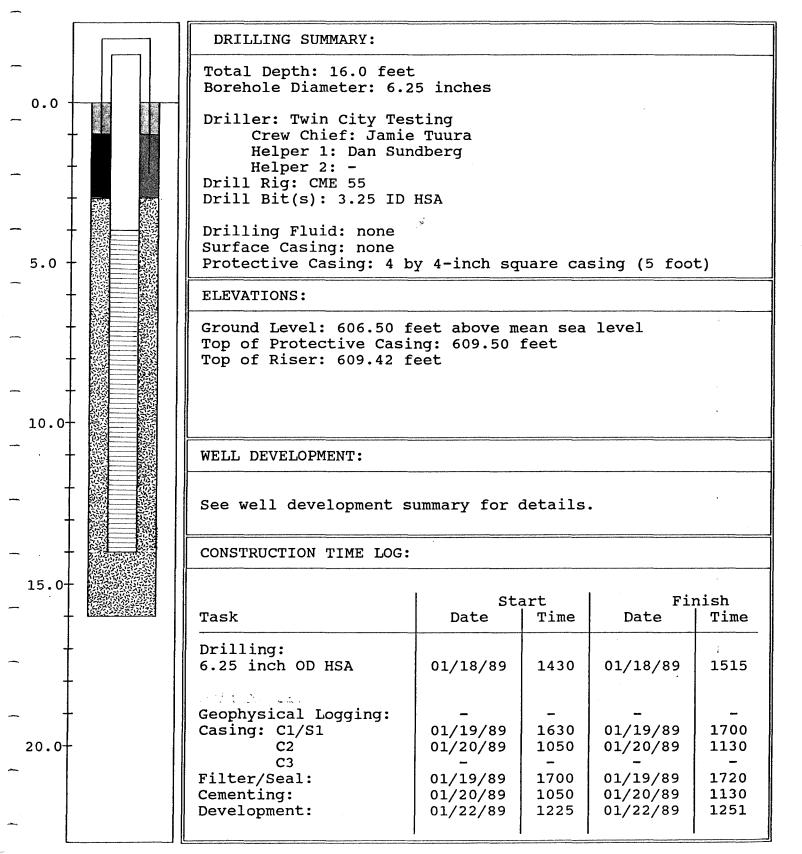
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Sheet 1 of 2

MONITORING WELL CONSTRUCTION SUMMARY

Project : City of Ashland (BRA10041)
Location: Ashland, Wisconsin
Survey Coordinates:
Borehole ID #: SB-1
Well ID #: MW-1
Supervised By: J.C. Dellaport



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Sheet 2 of 2

MONITORING WELL CONSTRUCTION SUMMARY

- Project : City of Ashland (BRA10041)	Borehole ID #: SB-1
Location: Ashland, Wisconsin	 Well ID # : MW-1
Survey Coordinates:	Supervised By: J.C. Dellaport

WELL DESIGN AND SPECIFICATIONS: Basis: Geologic Log Casing String(s): Depth (feet) Casing String +3.0 - 4.0**C**1 **S**1 4.0 - 14.0+3.0 - 2.0C2 C = CasingS = ScreenTwo inch diameter, threaded flush joint, stainless steel, Casing: C1: 7 foot length, O-ring joints ("Romac 4"). C2: Four inch square, 5 foot length, primer gray steel casing with hinged cap. Screen: S1: Two inch diameter, threaded flush joint, stainless steel, 0.010 inch continuous slot, 10 foot section, O-ring joints. Filter Pack: Medium-Coarse Rounded Flint Sand: 3.0 - 14.0 feet ¹₂-inch Bentonite Tablets: 2.0 - 3.0 feet Annular Seal: Dry Bentonite Powder: 1.0 - 2.0 feet Backfill: None Surface Seal: Concrete [portland cement, sand, dry bentonite powder, (10:1:0.5)]: 0.3 - 1.0 feet Other: Two inch vented PVC riser slip cap. Concrete guard posts. COMMENTS: Permanent ground-water quality monitoring well. The bentonite pellets were hydrated with one gallon of distilled water 24 hours prior to installation of the surface seal to ensure that the cement mix would not migrate past the annular seal and into the sand pack.

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Sheet 1 of 2

By: J.C. Dellaport

MONITORING WELL CONSTRUCTION SUMMARY

 Project : City of Ashland (BRA10041)		Borehole ID #: SB-2
Location: Ashland, Wisconsin	. 	Well ID # : MW-2
Survey Coordinates:		Supervised By: J.C.

DRILLING SUMMARY: Total Depth: 16.0 feet Borehole Diameter: 6.25 inches 0.0 Driller: Twin City Testing Crew Chief: Jamie Tuura Helper 1: Dan Sundberg Helper 2: -Drill Rig: CME 55 Drill Bit(s): 3.25 ID HSA ્ય Drilling Fluid: none Surface Casing: none Protective Casing: 4 by 4-inch square casing (5 foot) 5.0 ELEVATIONS: Ground Level: 606.43 feet above mean sea level Top of Protective Casing: 609.32 feet Top of Riser: 609.32 feet 10.0-WELL DEVELOPMENT: See well development summary for details. CONSTRUCTION TIME LOG: 15.0 Finish Start Time Date Time Task Date Drilling: 01/18/89 1615 1530 6.25 inch OD HSA 01/18/89 Geophysical Logging: 01/20/89 0930 01/20/89 0910 Casing: C1/S1 01/20/89 1030 C2 01/20/89 1000 20.0-**C**3 -01/20/89 1000 0930 01/20/89 Filter/Seal: 01/20/89 1030 01/20/89 1000 Cementing: 1154 1052 01/22/89 Development: 01/22/89

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Sheet 2 of 2

MONITORING WELL CONSTRUCTION SUMMARY

Project : City of Ashland (BRA10041)	Borehole ID #: SB-2
Location: Ashland, Wisconsin	Well ID # : MW-2
Survey Coordinates:	Supervised By: J.C. Dellaport

WELL DESIGN AND SPECIFICATIONS: Basis: Geologic Log Casing String(s): Depth (feet) Casing String +3.0 - 3.5C1 3.5 - 13.5**S**1 +3.0 - 2.0C2 ÷., C = CasingS = ScreenTwo inch diameter, threaded flush joint, stainless steel, Casing: C1: 6.5 foot length, O-ring joints ("Romac 4"). C2: Four inch square, 5 foot length, primer gray steel casing with hinged cap. S1: Two inch diameter, threaded flush joint, stainless steel, Screen: 0.010 inch continuous slot, 10 foot section, o-ring joints. Filter Pack: Medium-coarse Rounded Flint Sand: 2.5 - 13.5 feet Annular Seal: Dry Bentonite powder: 1.0 - 2.5 feet Backfill: None Surface Seal: Concrete [portland cement, sand, dry bentonite powder, (10:1:0.5)]: +0.3 - 1.0 feet Other: Two inch vented PVC riser slip cap. Concrete guard posts. COMMENTS: Permanent ground-water quality monitoring well. Used bentonite powder rather than tablets as annular seal to facilitate the installation of the protective casing to 2 feet below ground surface.

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Sheet 1 of 2

MONITORING WELL CONSTRUCTION SUMMARY

Project : City of Ashland (BRA10041) Location: Ashland, Wisconsin Survey Coordinates: Borehole ID #: SB-3 Well ID # : MW-3 Supervised By: J.C. Dellaport

DRILLING SUMMARY: Total Depth: 16.0 feet Borehole Diameter: 6.25 inches 0.0 Driller: Twin City Testing Crew Chief: Jamie Tuura Helper 1: Dan Sundberg Helper 2: -Drill Rig: CME 55 Drill Bit(s): 3.25 ID HSA Drilling Fluid: none Surface Casing: none Protective Casing: 4 by 4-inch square casing (5 foot) 5.0 ELEVATIONS: Ground Level: 610.37 feet above mean sea level Top of Protective Casing: 613.22 feet Top of Riser: 613.26 feet 10.0-WELL DEVELOPMENT: See well development summary for details. CONSTRUCTION TIME LOG: 15.0-Finish Start Time Date Time Task Date Drilling: 6.25 inch OD HSA 01/19/89 1308 01/19/89 1530 and the second Geophysical Logging: Casing: C1/S1 01/19/89 01/19/89 1530 1605 20.0-C2 01/20/89 1600 01/20/89 1613 **C**3 Filter/Seal: 01/19/89 1605 01/19/89 1620 Cementing: 01/20/89 1600 01/20/89 1613 01/22/89 01/22/89 0930 0944 Development:

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Sheet 2 of 2

MONITORING WELL CONSTRUCTION SUMMARY

Project : City of Ashland (BRA10041)Borehole ID #: SB-3Location: Ashland, WisconsinWell ID # : MW3Survey Coordinates:Supervised By: J.C. Dellaport

WELL DESIGN AND SPECIFICATIONS: Basis: Geologic Log Casing String(s): Depth (feet) Casing String +3.0 - 5.0C1 5.0 - 15.0**S1** +3.0 - 2.0C2 C = CasingS = ScreenTwo inch diameter, threaded flush joint, stainless steel, Casing: C1: 8 foot length, O-ring joints ("Romac 4"). C2: Four inch square, 5 foot length, primer gray steel casing with hinged cap. Two inch diameter, threaded flush joint, stainless steel, Screen: S1: 0.010 inch continuous slot, 10 foot section, O-ring joints. Filter Pack: Medium-Coarse Rounded Flint Sand: 4.0 - 10.5 feet Natural Sand Pack: 10.5 - 15.0 feet Annular Seal: ¹/₂-inch Bentonite Tablets: 2.0 - 4.0 feet One Gallon Distilled Water Backfill: None Surface Seal: Concrete [portland cement, sand, dry bentonite powder, (10:1:0.5)]: 0.3 - 2.0 feet Other: Two inch vented PVC riser slip cap. Concrete guard posts. COMMENTS: Permanent ground-water quality monitoring well. Hole was augered with a 3.25-inch ID HSA (6.25 OD) to 16 feet. Well casing and screen was installed to 15 feet through auger. As auger was removed, caving of sand occured which provided a natural sand pack from 10.5 to 15.0 feet. Coarse filter sand was added to complete the sand pack from 4.0 to 10.5 feet. Well leans slightly to the north. Driller was unable to

orient tower to vertical since drilling site was sloped to north.

APPENDIX D

WELL DEVELOPMENT SUMMARIES

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WELL DEVELOPMENT SUMMARY

_	Project :	City of Ashland (BRA10041)	Well Number :	MW-1
	Location :	Ashland, Wisconsin	Well Location :	
	Personnel:	D.A. Rautmann, J.C. Dellaport	Riser Elevation :	609.42
_			Ground Elevation:	606.50

DATE 1989	TIME	METHOD	VOL gal	FINAL APPEAR	TEMP °C	pH su	CONDUCT µmho/cm	HNu ppm	COMMENTS
1/22	1225	C Pump	0						start pur
				2					yellowish
									soapy fil
				4					oil sheer
1/22	1226		5	4					*
1/22	1229		15	3					
1/22	1230		20	3					
1/22	1231		25	3					
1/22	1232		30	3					
1/22	1234				12	6.3	540		sample 1
1/22	1239				10	6.6	530		sample 2
1/22	1244				10	6.8	530		sample 3
1/22	1250		~130						stop pump
		-							
	SS H B Pu C Pu PC H PI H PR H	Bail : Sta	standa ainless arce or adder p ntrifug ses ristalt omerged rge pum	ard bailer s steel po standard oump gal pump w sic pump piston p p	oint bailer vith PVC pump		PPEARANCE: 1 = Clear 2 = Slight 3 = Cloudy 4 = Very C 5 = Cloudy 6 = Muddy 7 = Very M	2loudy y - Muo	-

* full throttle on pump

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WELL DEVELOPMENT SUMMARY

Project : City of Ashland (BRA10041)
 Location : Ashland, Wisconsin
 Personnel: D.A. Rautmann, J.C. Dellaport

Well Number : MW-2 Well Location : Riser Elevation : 609.32 Ground Elevation: 606.43

_	DATE 1989	TIME	METHOD	VOL gal	FINAL APPEAR	TEMP °C	pH su	CONDUCT µmho/cm	HNu ppm	COMMENTS
	1/22	1052	C Pump	0	7			<u> </u>		start pump
	1/22	1054		2.5	7					w.l.= 7.2
	1/22	1056		5	7					w.l.= 7.27
	1/22	1058		10	6					stop pump
_						~2 [°]				
	1/22	1117		0						start pump
~	1/22	1119		о	3					discharge
	1/22	1120		5	3					*
-	1/22	1121		10	3					**
	1/22	1122		15	3					***
	1/22	1127		50	2					
-	1/22	1140			.1	10	5.7	660		
	1/22	1141			1	12	5.5	680		
-	1/22	1144			1	10	5.5	620		
	1/22	1152								stop pump
	1/22	1152		~170						w.l.= 7.32
-	-			-						
	METHOD = PVC Bail: PVC Point source or or standard bailerAPPEARANCE:SS Bail : Stainless steel point source or standard bailer1 = Clear 2 = Slightly Cloudy 3 = CloudyB Pump : Bladder pump C Pump : Centrifugal pump with PVC hoses3 = Cloudy 4 = Very Cloudy 5 = Cloudy - Muddy 6 = MuddyPC Pump : Peristaltic pump PI Pump : Submerged piston pump PR Pump : Purge pump7 = Very MuddyCONDUCT = Specific Conductance (µmho/cm) at ambient temperature7 = Very Muddy									

* slight sewage odor ** light brown to yellow *** oily soap bubbles

WELL DEVELOPMENT SUMMARY

Project : City of Ashland (BRA10041)
 Location : Ashland, Wisconsin
 Personnel: D.A. Rautmann, J.C. Dellaport

Well Number : MW-3 Well Location : Riser Elevation : 613.26 Ground Elevation: 610.37

_	DATE 1989	TIME	METHOD	VOL gal	FINAL APPEAR	TEMP °C	pH su	CONDUCT µmho/cm	HNu ppm	COMMENTS			
	1/22	0930	C Pump		7					start pump			
-	1/22	0942		3	7					pumped dry			
										stop pump			
-	1/22	0943			7								
_	1/22	1025		4	7								
	1/22	1029			7								
-	NOTE: Due to natural sandpack from 10.5 - 15.0 feet, MW-3 must be												
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-													
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1	· .							n an an Alfred Martin ann an an Anna an Anna an Aird an Aird Alfred Martin Anna ann an Anna ann an Anna ann an					
	METHOD = PVC Bail: PVC Point source or APPEARANCE:												
	or standard bailer SS Bail : Stainless steel point 1 = Clear									;			
	source or standard baile B Pump : Bladder pump							3 = Cloudy					
-			imp : Cen hos		4 = Very Cloudy 5 = Cloudy - Muddy								
	PC Pump : Peristaltic pump6 = MuddyPI Pump : Submerged piston pump7 = Very Muddy												
-	CONDUC	PR F	Pump : Pur ecific Con	ge pum	ເp	-		A - 1	-				
			ient temp										

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APPENDIX E

FIELD WATER QUALITY SAMPLING AND ANALYSIS

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Sheet 1 of 1

FIELD WATER QUALITY SAMPLING AND ANALYSIS

Project : City of Ashland (BRA10041)
 Personnel : J. C. Dellaport
 Location : Wastewater Treatment Plant, Ashland, Wisconsin

	T	1	1	Т				
Location:	MW-1	MW-2	MW-3	Artesian				
Water Source:	SS Well	SS Well	SS Well	SS Well				
Date:	01/25/89	01/25/89	01/25/89	01/25/89				
Time:	1300	1335	1015	1000				
SAMPLING CONDITIONS:								
Sampling Method:	SS Bail	SS Bail	SS Bail	SS Bail				
Sampling Depth: (Feet)	7	7	11	unknown				
Well Depth: (Feet)	14.0	13.5	15.0	unknown				
Static Water Level: (Feet below measurement point)	7.28	7.19	11.12	+17 feet				
Final Purge Volume: (Gal)	3.0	3.0	3.0	flowing				
FIELD MEASURMENTS AND ANA	LYSIS:	·						
Temperature: (°C)	7.5	5.5	9	7				
Specific Conductance:	590	680	870	265				
(µmho/cm) at°C: Corrected to 25°C:	800	950	1150	360				
pH: *	9.1	9.0	8.9	6.7				
HNu: (ppm)	-	-	-	-				
Sampling Instruments: Temperature: Red Alcohol Thermometer Conductivity: Nester MHo Pen pH: Nester pH Pen PID: HNu Model PI101								
SAMPLES COLLECTED AND TREA	ATED:							
PCBs, BNAs, VOCs, Metals	Х	х	X	X				
Wastewater Parameters	х	х	X	X				
Laboratory: Enviroscan Date:	01/26/89	01/26/89	01/26/89	01/26/89				
ł		JCD	JCD	JCD				

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APPENDIX G

CONSTRUCTION DRAWINGS SHOWING CLAY WALL DETAIL

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